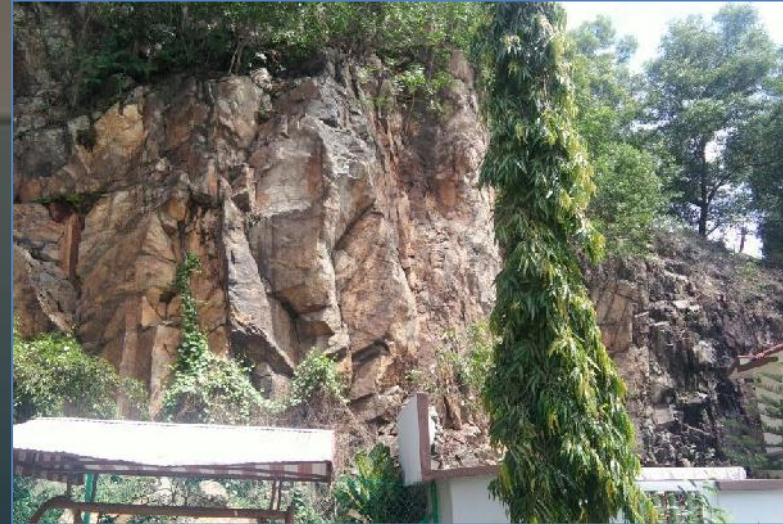


KURSUS PENSIJILAN CERUN TAHAP 2

GEOLOGY PART 2:

ROCK MASS DESCRIPTION & CHARACTERISATION, ROCK SLOPE STABILITY ANALYSIS & STABILIZATION AND PROTECTION METHODS



NURUL IFFAH ISMAIL (GEOLOGIST)

GEOTECHNICAL FORENSIC ENGINEERING, KUMPULAN IKRAM SDN BHD

16 MARCH 2020 (MONDAY)

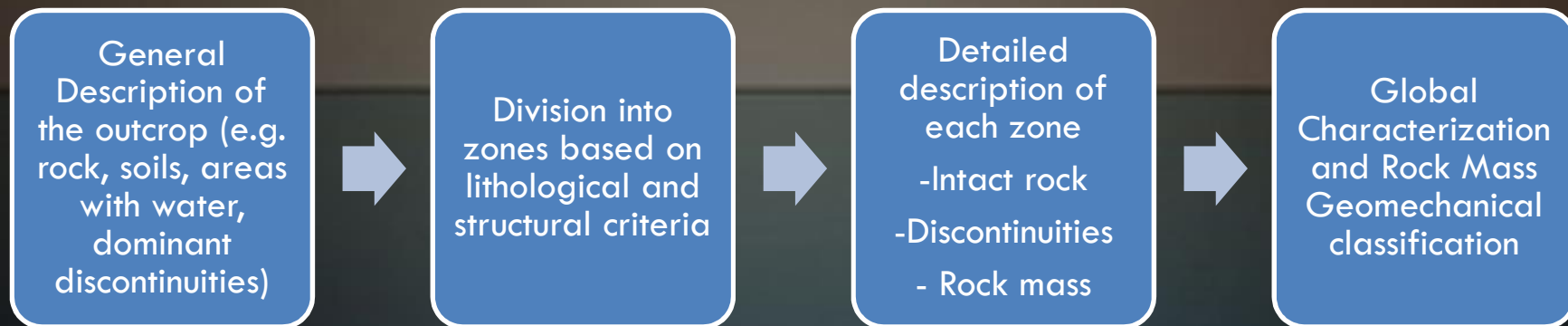
Pusat Kecemerlangan Kejuruteraan dan Teknologi JKR (CREaTE)

OUTLINE

1. Rock Mass Description & Characterization
2. Rock Mass Rating (RMR)
3. Rock slope stability analysis
4. Rock Slope Stabilization & Protection



DESCRIPTION OF ROCK MASS OUTCROP FLOW CHART





GEOLOGY AND ROCK PROFORMA

Cawangan Kejuruteraan Geoteknik

TITLE OF PROJECT		MEMBANGUNKAN SISTEM APLIKASI HAZARD AND RISK MAP KUALA LUMPUR UNTUK DEWAN BANDARAYA KUALA LUMPUR					
ZONE	1/2/3/4	PARTILMENT	DBKL ZONING	T.S.E NO	SLOPE NO		
POSCODE	TAMAN	ROAD NAME	ROUTE NO	SECTION NO	SECTION NO		
ROAD LEFT	RIGHT	DISTANCE FROM KM POST	AGE	(year built / year)	SLOPE ID		

FILL IN BOXES OR CIRCLE CORRECT ANSWER

STEP 1 INSPECTION DETAILS

1.1 INSPECTED BY							
1.1 TEAM	TEAM 1	TEAM 2	TEAM 3	TEAM 4	TEAM 5	TEAM 6	TEAM 7
1.2 DATE				1.3 TIME			

STEP 2 GENERAL INFORMATION

2.1 SECTION NO.		2.2 DISTANCE	(m from km post)	2.3 SLOPE FACE / FEATURE ASPECT	Dip / Dip direction	(m to step 4)	
2.4 COORDINATE:	NORTH	EAST					
(WGS) (Position Of Slope ID Marker)							

STEP 3 GEOLOGY AND STRUCTURAL

3.1 ROCK TYPE				3.2 WEATHERING GRADE (SCALE 1 TO 6)			
3.3 GEOLOGICAL FEATURE:	Jointed (J)	Faulted (F)	Uncertainty (U)	Schistosity (S)	Intrusion Zone (I)	Sheared / crushed zone (Sp)	Foliation (F)
	Bedding (B)	Folded (F)	Clauage (C)	Overhang (O)	Tension crack (T)	Ravelling (heavily jointed & fragile material) (R)	Fracture (F)
3.4 ADVERSE GEOLOGICAL FEATURES/EVIDENCE OF DISTRESS				3.5 EVIDENCE OF PAST FAILURE			
1	No evidence of surficial loosening			1	No recorded or observed evidence of past instability		
2	Localised surficial loosening, or small overhanging blocks			2	Observed evidence of past instability (rock blocks and fragments accumulated at toe of slope)		
3	Evidence of a fault line, bedding, fracture planes, daylighting joints or tension cracks exist along crest of slope.			3	Documented evidence of past instability - Minor failure (volume < 50m ³)		
4	Surficial loosening and small overhanging blocks in several areas of slope			4	Documented evidence of past instability - Moderate failure (50m ³ < Volume < 500m ³)		
5	Large overhanging blocks with potential release surfaces visible			5	Documented evidence of past instability - Major failure (volume > 500m ³)		

Please use these ABBREVIATION for Step 4

Specimen size (mm)	Permeability (m)	Termination	Roughness	Seepage	Infilled
SC 1-20	VL 1-1	R Outside exposure	R Rough	1 Not possible	B (all)
VC 20-40	L 1-3	R Within exposure	S Smooth	2 Dry	V Veneer
C 60-200	M 3-10	D Against another	P Polished	3 Dry but show evidence	S Stain
M 200-600	H 10-20	Shape	K Discontinuity	4 Damp	Cm Cemented
W 600-2000	VH 20	p Single Plane	Infilled strength	5 Dipping (Flow in Limestone)	T Trace
VW 2000-6000		S Several Plane	W Weak Material	6 Flowing (Limestone)	Cn Clean
FW >6000		U Irregular/Wavy	S Intact Material		

STEP 4 DISCONTINUITY INFORMATION (30 reading/100m slope length)

* note : please indicate Chainaging (horizontal distance from Slope ID Marker)

No	Feature type (see 3.3)	Dip/Dip direction	Chainaging	Spacing	Persistence	Termination	Shape	Aperture Range (mm)	Roughness	Seepage*	Infilled/Strength	Remarks
SCAN LINE 1 (Height = in from Finish Road Level)												
1												
2												
3												
4												
5												
6												
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												

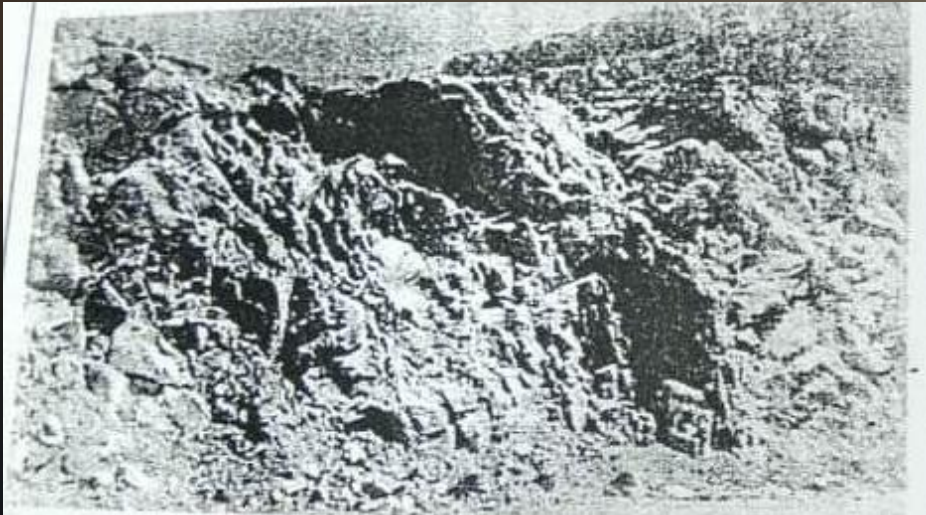
Note: (1) Please proceed the "Discontinuity Information" with new Scan Line on a new sheet if required. (2) Record 50 informations for every 100m run slope length. (3) Please set the Scan Line at the base, middle and top of the slope.

JKR Pre-Construction

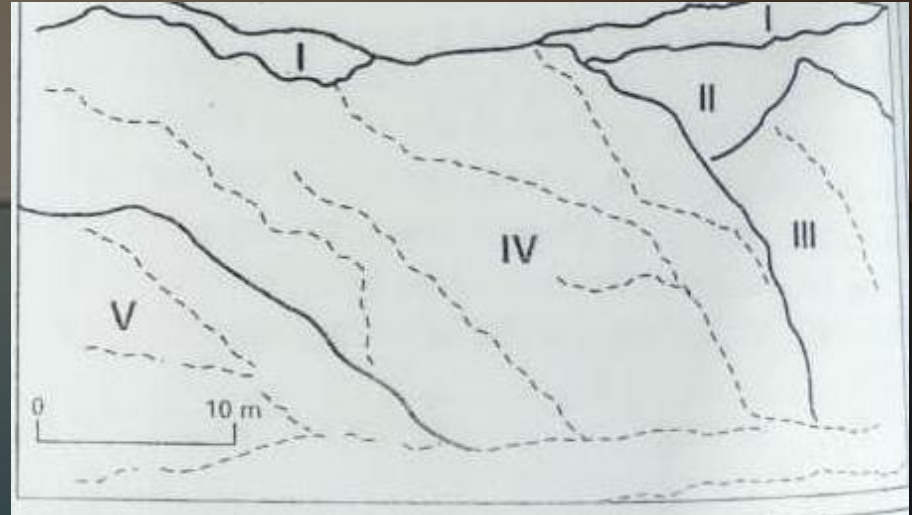
GEOLOGY PROFORMA

DIVISION INTO ZONES AND GENERAL DESCRIPTION OF EACH ZONE

PHOTO



SKETCH





DETAILED INVESTIGATION BY GEOLOGICAL MAPPING AND DISCONTINUITY SURVEYS



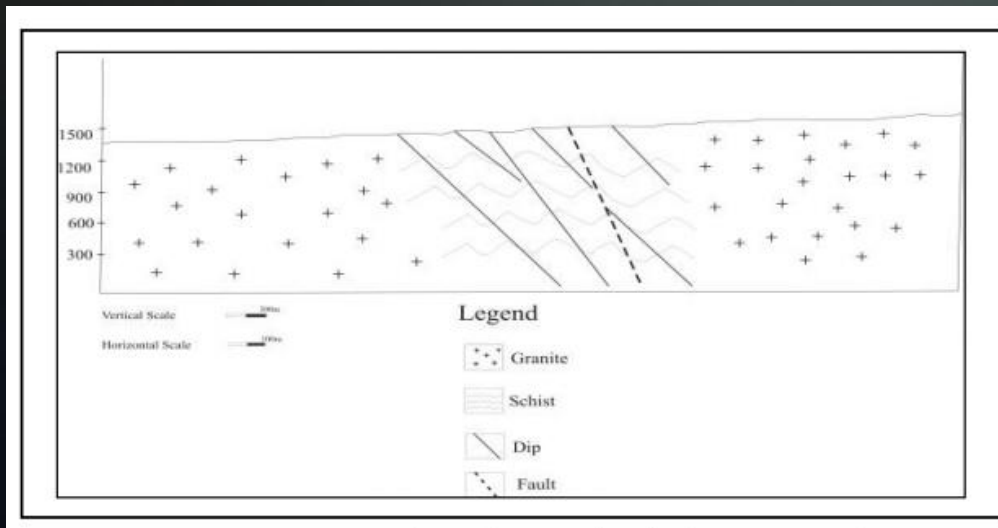
OUTPUTS FROM GEOLOGICAL MAPPING & DISCONTINUITY SURVEY

Production of geological models

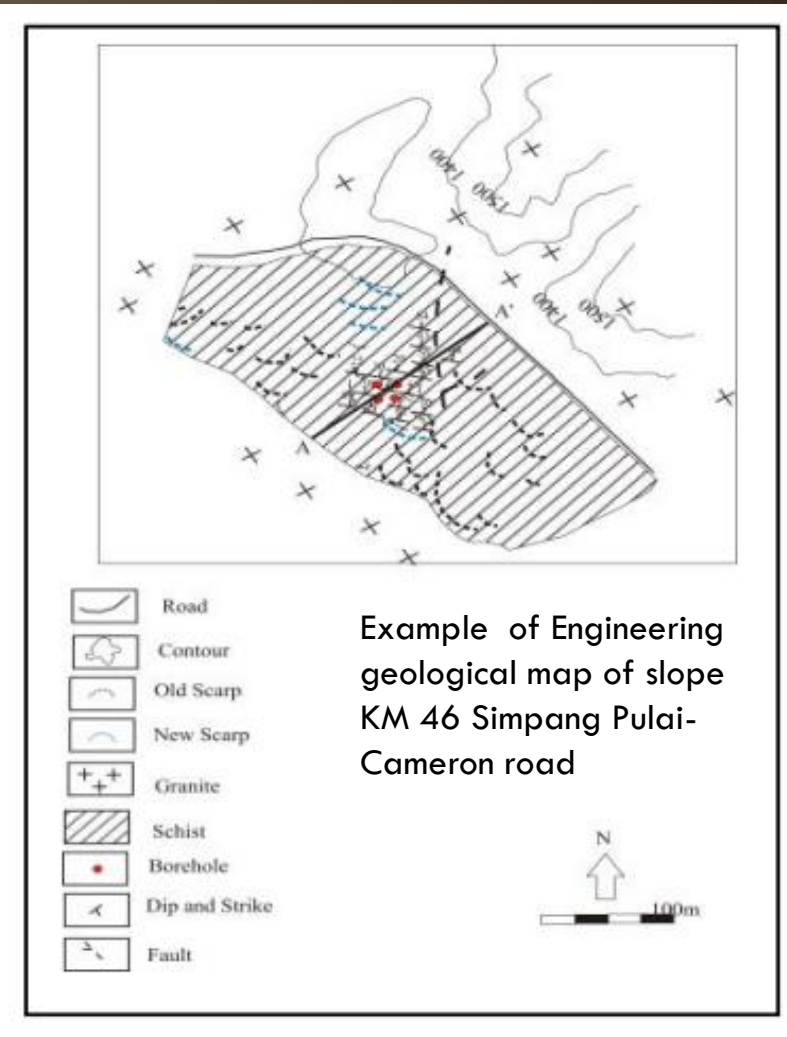
1.Engineering geological map

2.Field sketches

3.Cross sections



Engineering geological cross section



Example of Engineering geological map of slope KM 46 Simpang Pulai-Cameron road

- The strength of a large piece of rock is usually weaker **than** the strength of the small pieces of rock tested in the lab
- This is due to the presence of large scale discontinuities



Discontinuities



- Foliation



Bedding Plane & Joints



Fault & bedding plane

Discontinuities



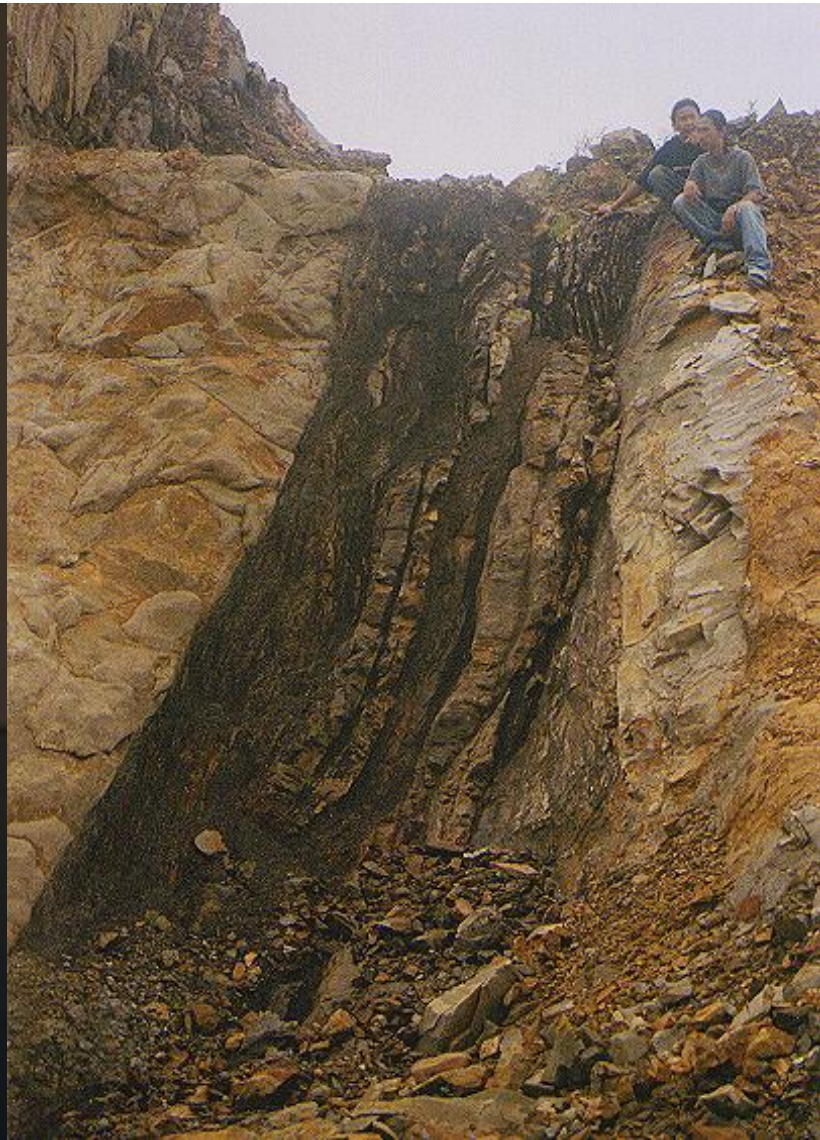
Fault (Thrust)



Thrust Fault



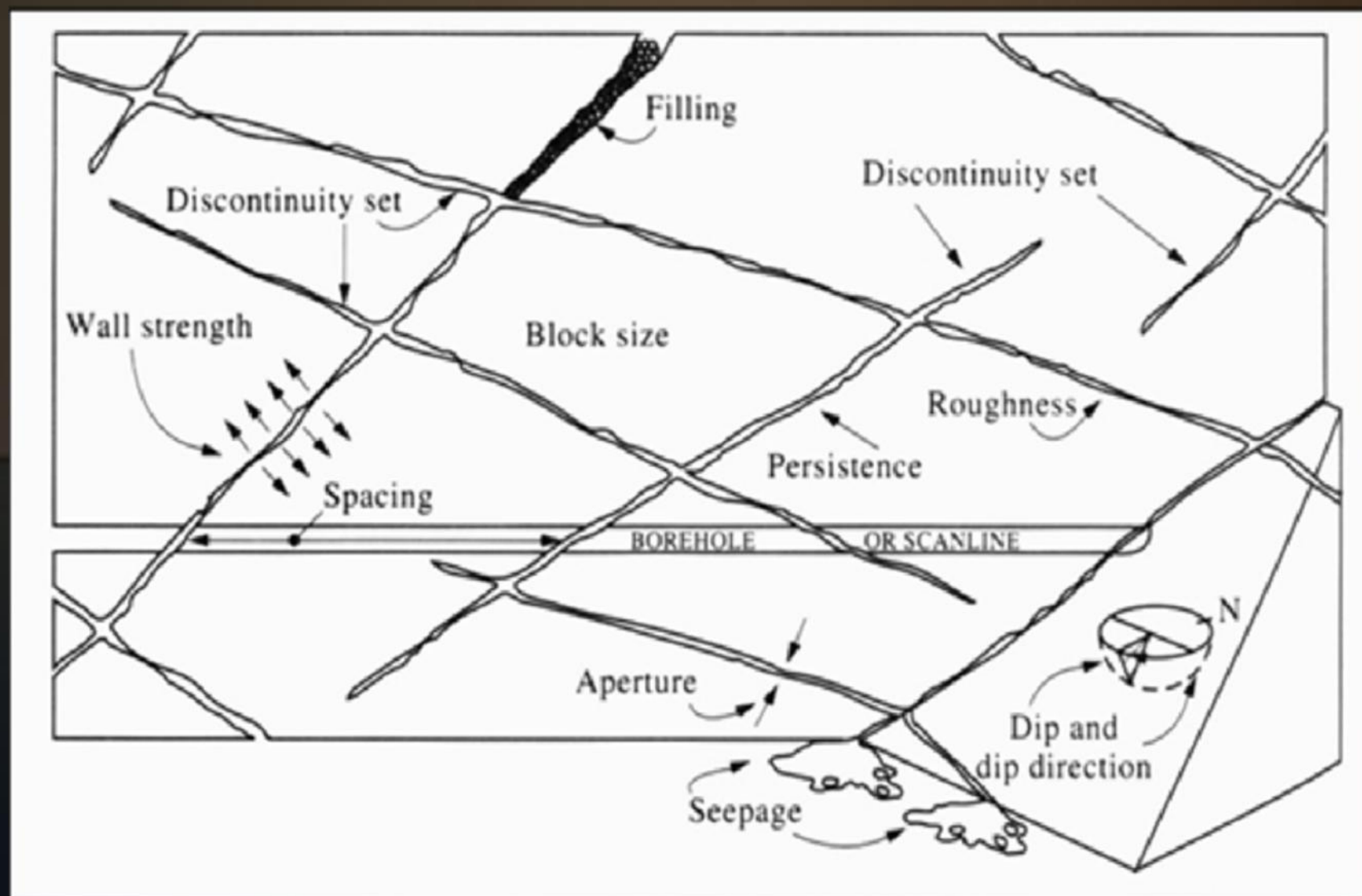
Shear zone



Shear Zone

Major fault





DESCRIPTION OF DISCONTINUITIES

GEOMETRY

1.Orientation

2.Spacing

3.Persistence

SHEAR STRENGTH

4.Roughness

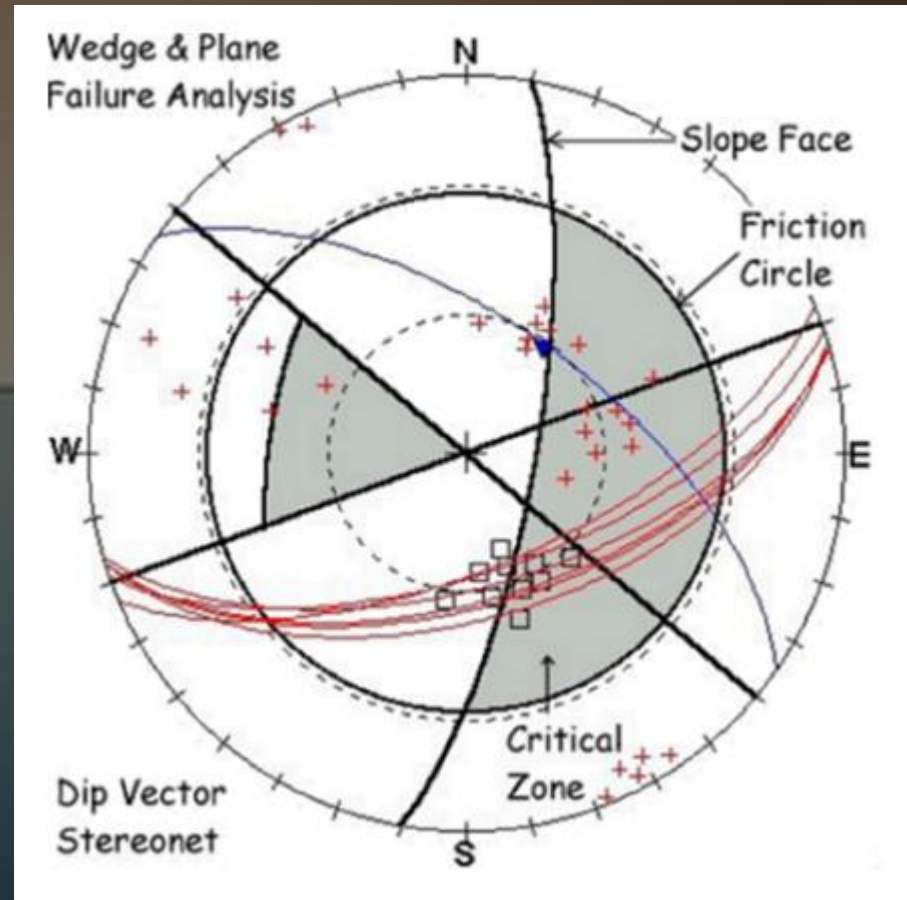
5. Aperture

6. Filling

7.Seepage (water)

1. ORIENTATION

- Using compass
- Dip and dip direction of the plane
- Plot using Kinematic stereonet analyses for rock slope stability - 'ROCKPACK III'
- – Enable the number of joint sets
- – Mean orientation



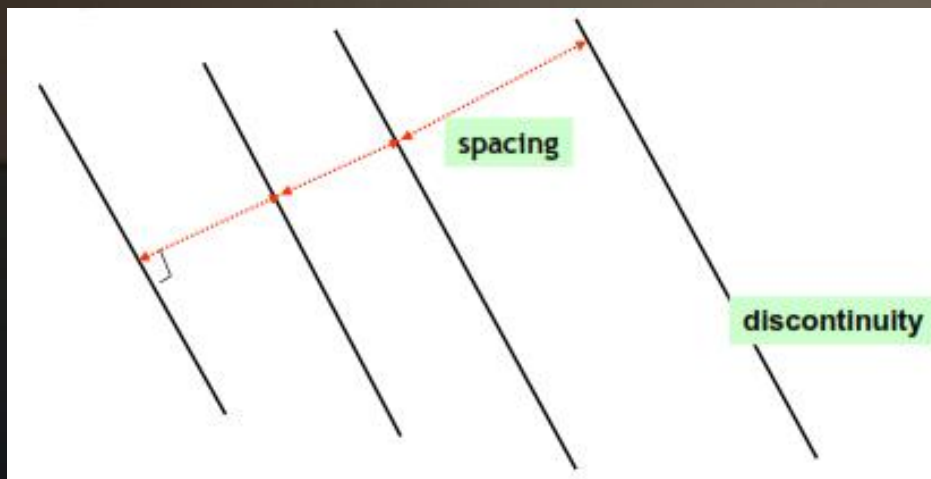
DIP AND DIP DIRECTION



2.SPACING

(how far apart the discontinuities are)

- The perpendicular distance between adjacent discontinuities



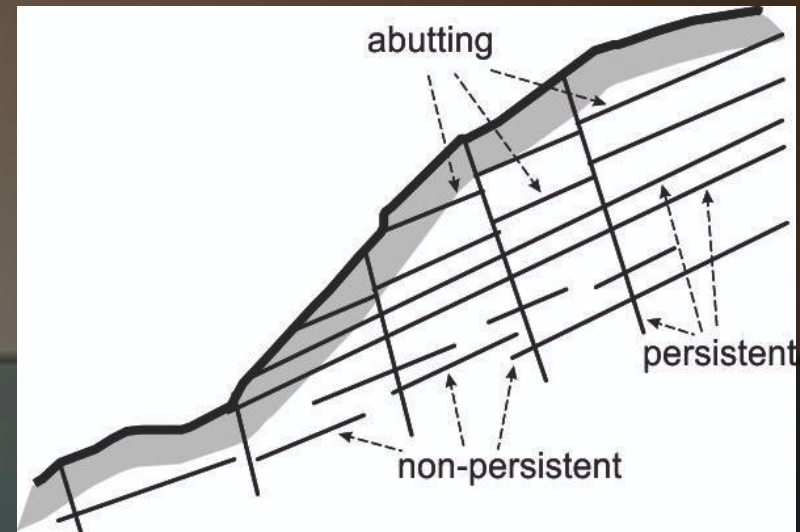
Description	Spacing
Extremely narrow spacing	< 2 cm
Very close spacing	2-6 cm
Narrow spacing	6-20 cm
Moderate spacing	20-60 cm
Wide spacing	60-200 cm
Very wide spacing	200-600 cm
Extremely large spacing	> 600 cm

ISRM classification according to the spacing

3. PERSISTENCE

(how continuous are they)

- A measure of the areal extent of a discontinuity
 - Inception to termination in solid rock or against another discontinuity
- Major joints;
 - the plane may extend beyond the limits of the exposure
 - Measure the maximum trace length



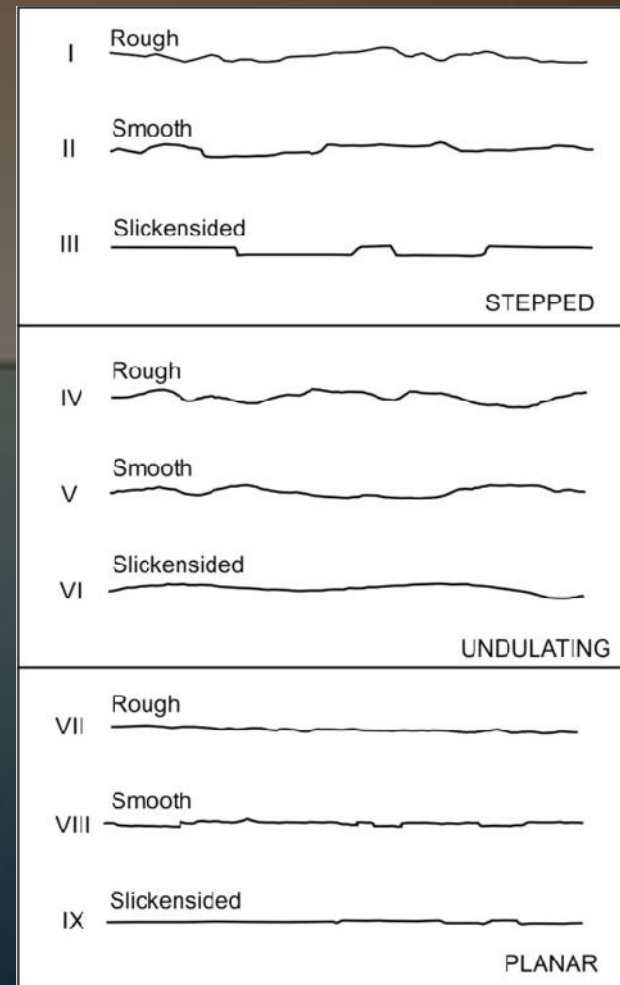
Description	Persistence
Very low persistence	<1 m
Low persistence	1-3 m
Average persistence	3-10 m
High persistence	10-20 m
Very high persistence	>20 m

ISRM classification according to the persistence

4. ROUGHNESS

(rough to smooth)

- Discontinuity surface
 - Stepped
 - Undulating
 - Planar
- Based on two scales:-
 - Small scale: several centimeters
 - Large scale: several meters
- The roughness gives indications for the evaluation of the shear strength of not filled discontinuities



Strength
Decreases

Typical ISRM surface roughness profiles.

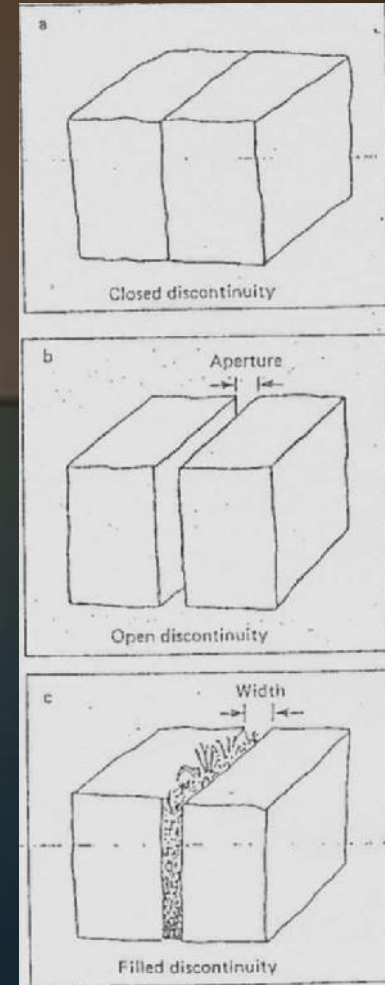
5.APERTURE

(is the discontinuity open or closed)

- The mean perpendicular distance between adjacent walls of a discontinuity.
- Space between walls that is filled with air or water

Aperture	Description	Discontinuity
<0.1 mm	Very narrow	Closed
0.1-0.25 mm	Narrow	
0.25-2.5 mm	Partly opened	
0.5-2.5 mm	Opened	Semi-open
2.5-10 mm	Moderately large	
>10 mm	Large	
1-10 cm	Very large	Open
10-100 cm	Extremely large	
> 1 m	Cavernous	

ISRM classification according to aperture



6.FILLING

(is the discontinuity empty or filled and with what?)

- The infilling between discontinuity
- surfaces
- • The infilling may be:
 - Soil introduced into the opening
 - Minerals such as calcite
 - Faults clay gouge (deep hole)
 - Breccia
- • Must be identified and described
- • The strength of the infill must be assessed
- visually and manually.

7. SEEPAGE (water)

- Water flow
- Free moisture
- Describe and rate of flow
 - estimated (liter/hr)

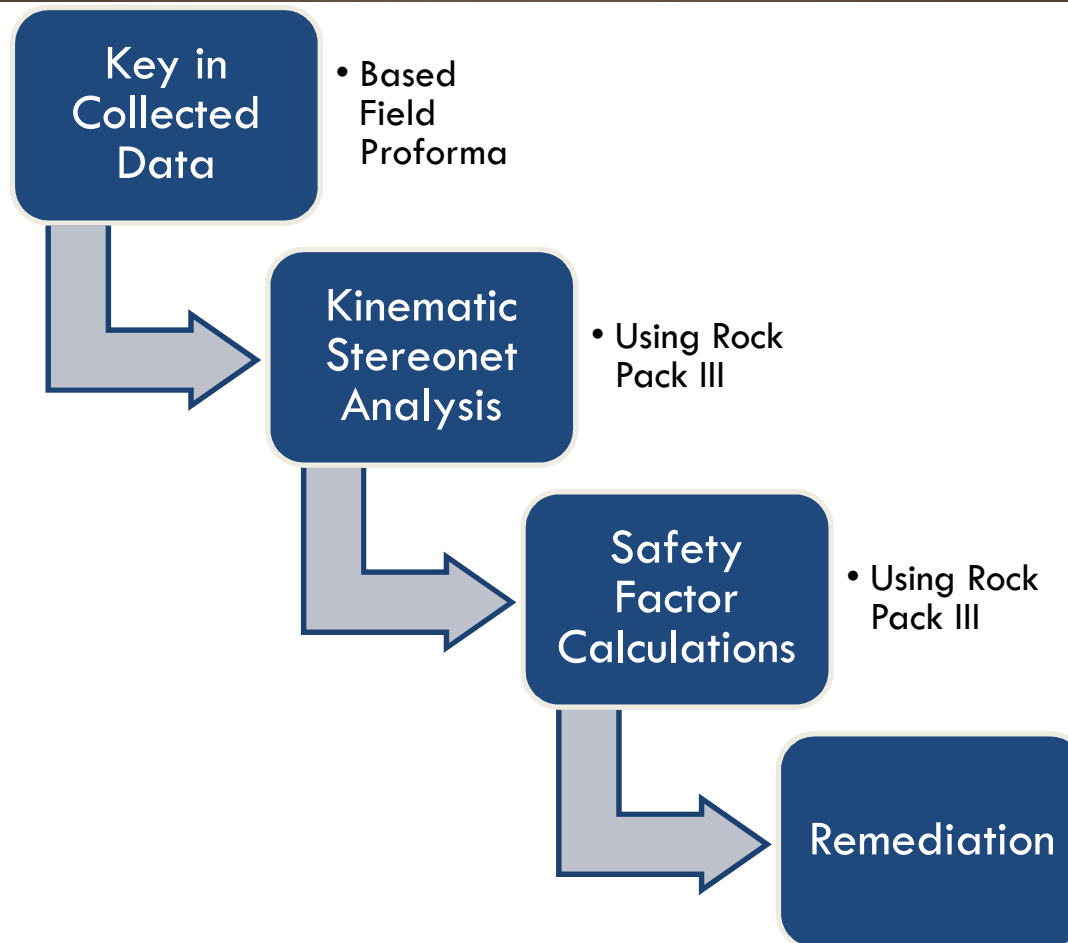


ROCK MASS RATING (RMR)

- SIX parameters are required:
 1. Uniaxial Compressive Strength (J_{A1})
 2. Rock Quality Design, RQD(J_{A2})
 3. Discontinuity spacing(J_{A3})
 4. Conditions of the discontinuities(J_{A4})
 5. Groundwater (flow and general condition)(J_{A5})
 6. Rating adjustment for discontinuity orientations(J_B)
- Based on ratings (total of 6 ratings)
- Total ratings (0-100) will give rock mass classes (I to V).
- Higher rating – better rock
 - **$RMR = JA1 + JA2 + JA3 + JA4 + JA5 + JB$**

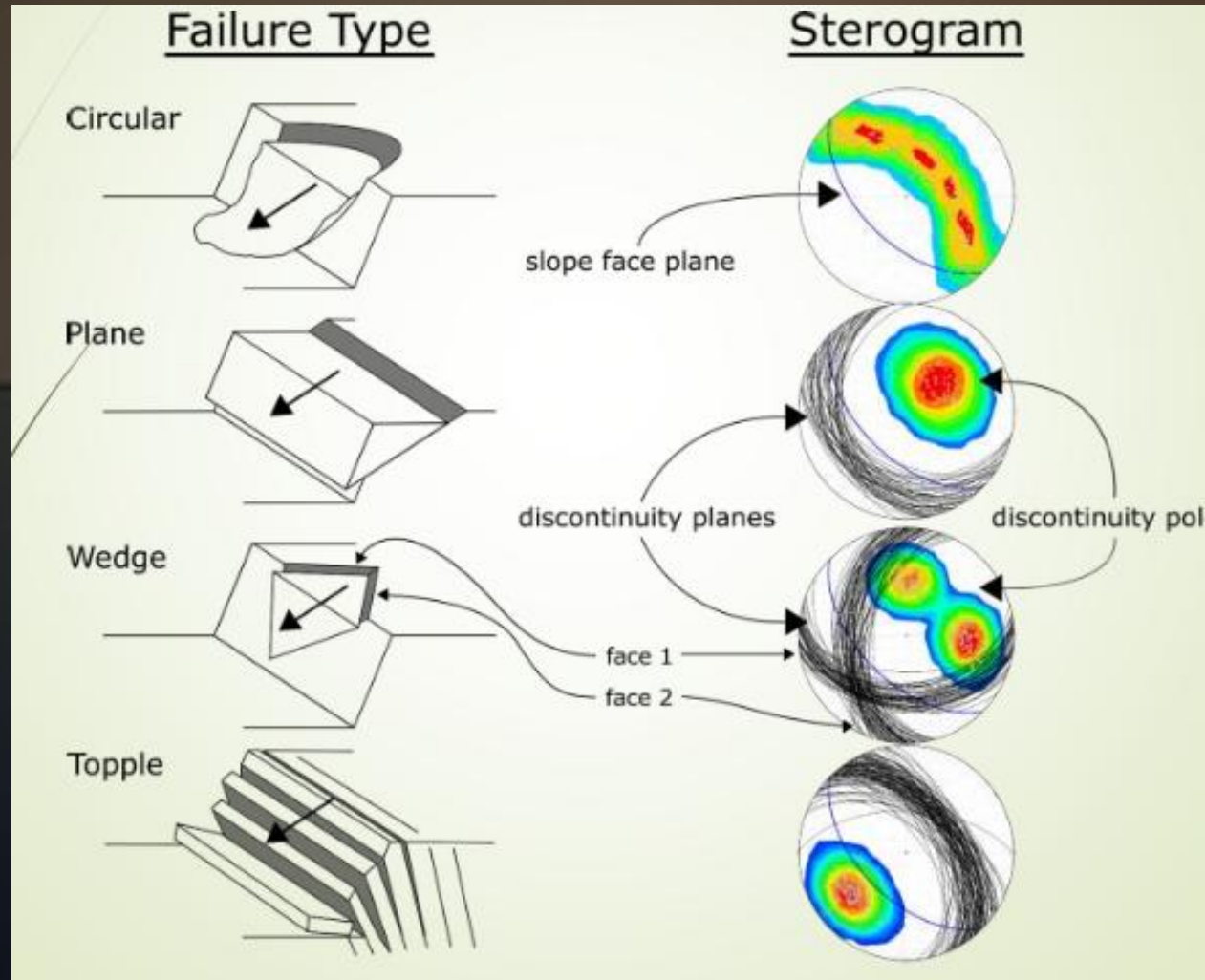
ROCK SLOPE STABILITY ANALYSIS

FLOW CHART OF ROCK SLOPE STABILITY ANALYSIS



A.KINEMATIC STEREONET ANALYSIS

STEREONET BASED KINEMATIC ANALYSIS



Rockpack III - Beta

File Edit View Tools Window Help



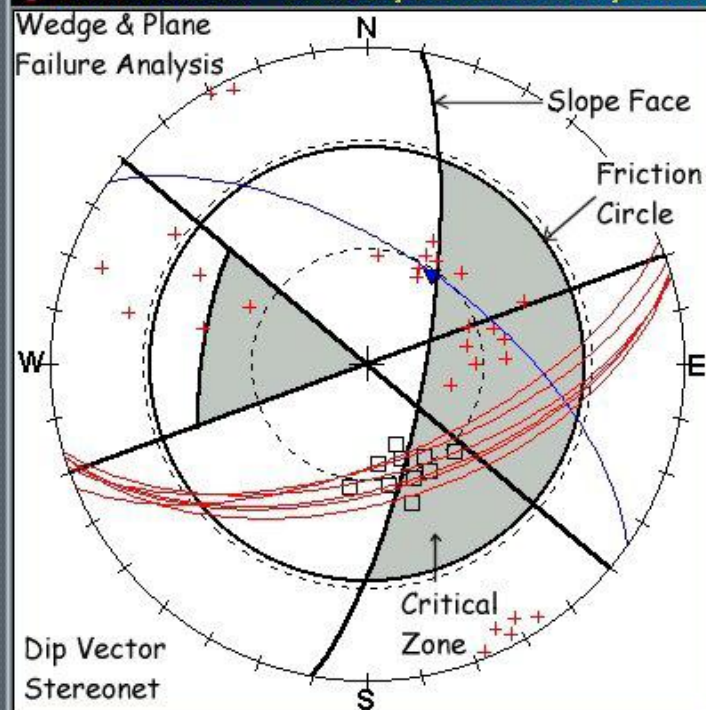
Rockpack III Data Entry

Data Set	Discontinuity	Traverse Trend	Distance	Structure	Dip Direction	Dip	Rock Type	Hardness	Water	Length	Continuity	Roughness	Spacing	Filling Type	Filling Hardness	Zone Width	Zone Sp
1	201	345	009	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	202	345	015	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	203	345	050	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	204	345	068	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	205	345	091	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	206	345	185	04	156	47	1	11	6	1000	1	2	00	001	00	00	
1	207	345	142	05	156	47	1	11	5	1000	1	2	00	187	00	00	
1	208	345	004	02	064	85	1	11	4	0005	1	2	00	003	00	00	

Stereonet Str...

- ☐ 1 - Bedding
- ☐ 2 - Small Joints
- ☐ 3 - Large Joints
- ☐ 4 - Sealed Joints
- ☐ 5 - Foliations
- ☐ 6 - Faults
- ☐ 7 - Not Defined

Stereonet Plot - C:\Rockpack\Data\newprob.csv



Dip Vector Plot

☒ Plot Great Circles

☐ Markland's Test

Friction Angle (0-90)

Dip Direction (0-360)

Dip Value (0-90)

☒ Topple

Draw

New

Open Data

☒ Define Structures

Structure Key

Save BMP

Dip Direction: 113

Dip Value: 8

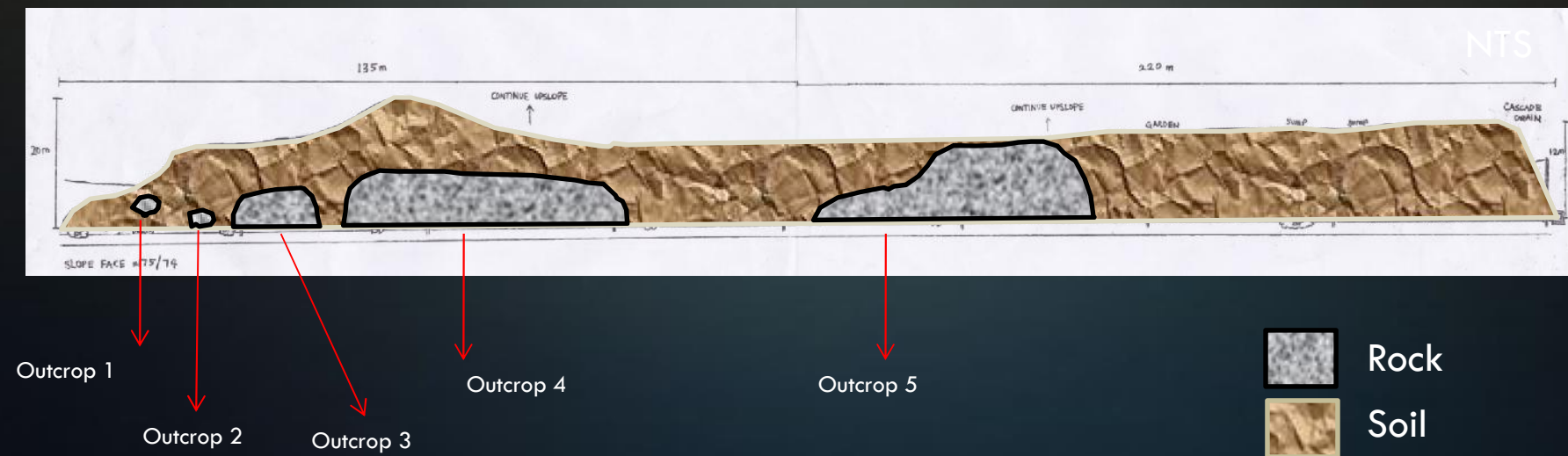
Number of Readings: 40

CASE STUDY BUKIT ANGGERIK

1. Site Sketch
2. Geomorphological Map
3. Stereonet Analysis

Site Sketch

ROCK AND SOIL BOUNDARY





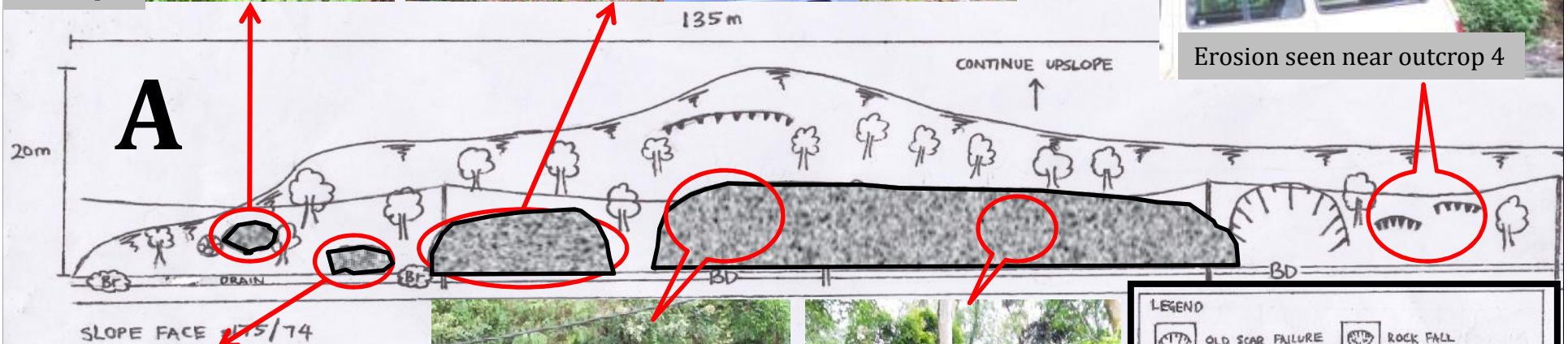
Outcrop 1



Outcrop 3



Erosion seen near outcrop 4



Outcrop 2



Rock fall



Tree roots penetrated rock outcrop

LEGEND	
	OLD SCAR FAILURE
	EROSION
	BROKEN DRAIN
	CRACK
	BLOCKED DRAIN
	ROCK FALL
	SEEPAGE
	GRANITE
	UP ROOTED TREE
	JOINT

B



Rock Outcrop 5

220 m

CONTINUE UPSLOPE

GARDEN

SUMP

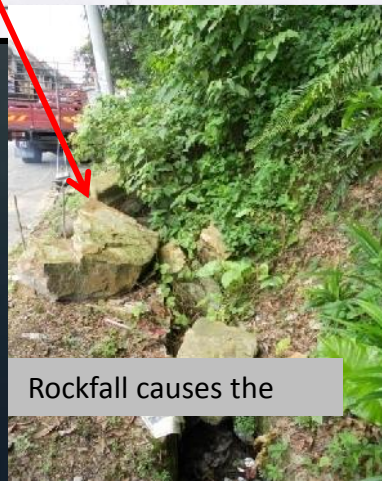
SUMP

CASCADE DRAIN

12m



Evidence of seepage



Rockfall causes the

LEGEND



OLD SCAR FAILURE



EROSION



BROKEN DRAIN



CRACK



BLOCKED DRAIN



ROCK FALL



SEEPAGE



GRANITE

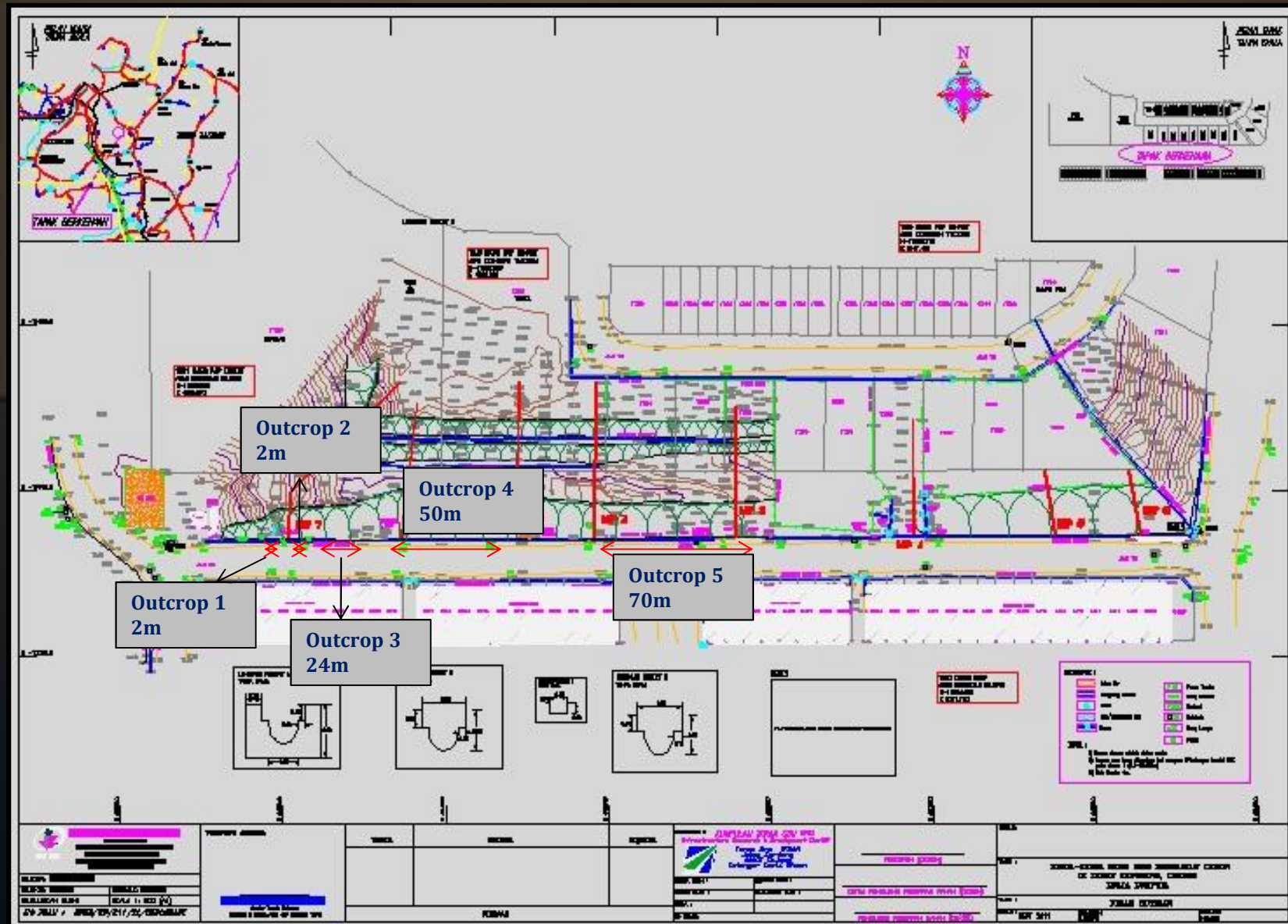


UP ROOTED TREE



JOINT

LAYOUT FOR STEREONET ANALYSIS



EXPOSED ROCK OUTCROP AT THE FAILURE AREA



Potential Rock Failure Seen on Site



STEREONET PLOT FOR OUTROP 1 (LENGTH: 2M)

Orientation of Slope Face:
56/176

Set of joints:
(J1) 68/043
(J2) 81/207

Toppling Failure Zone

Plane/Wedge Failure Zone

LEGEND:

▲ Joint/shear plane

INTERPRETATION:

NO POTENTIAL FAILURE

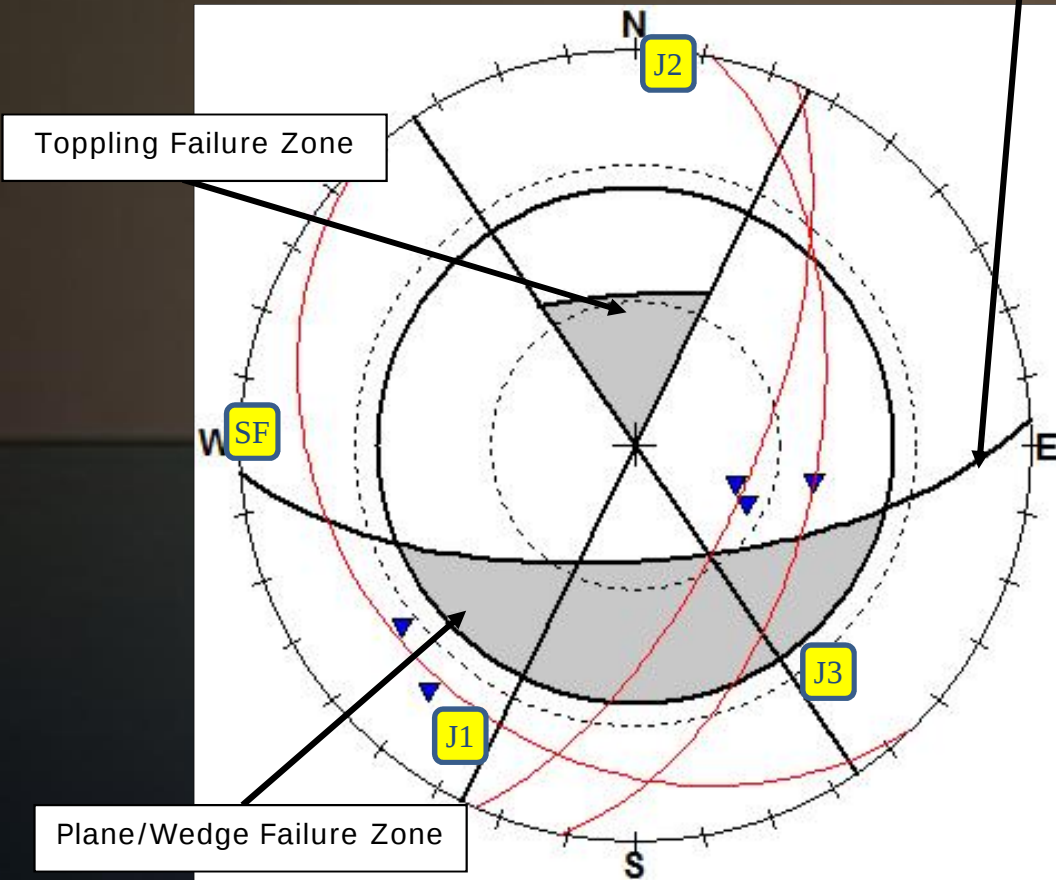


STEREONET PLOT FOR OUTROP 2 (LENGTH: 2M)

Orientation of Slope Face:
66/176

Set of joints:
(J1) 66/114
(J2) 52/101
(J3) 24/226

INTERPRETATION:
NO POTENTIAL FAILURE



LEGEND:

▲ Joint/shear plane



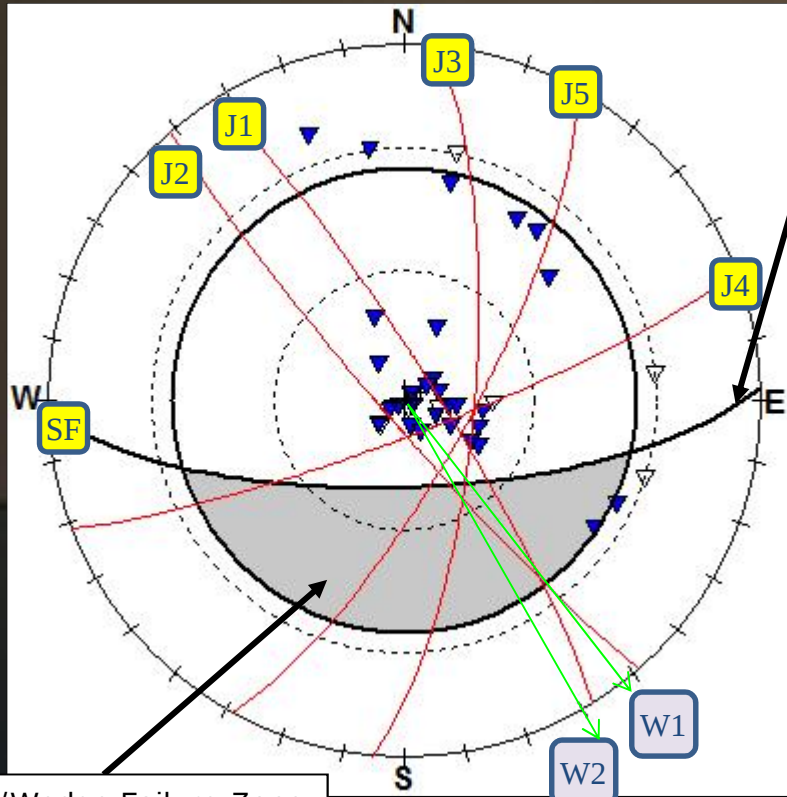
Picture Of Outcrop 3



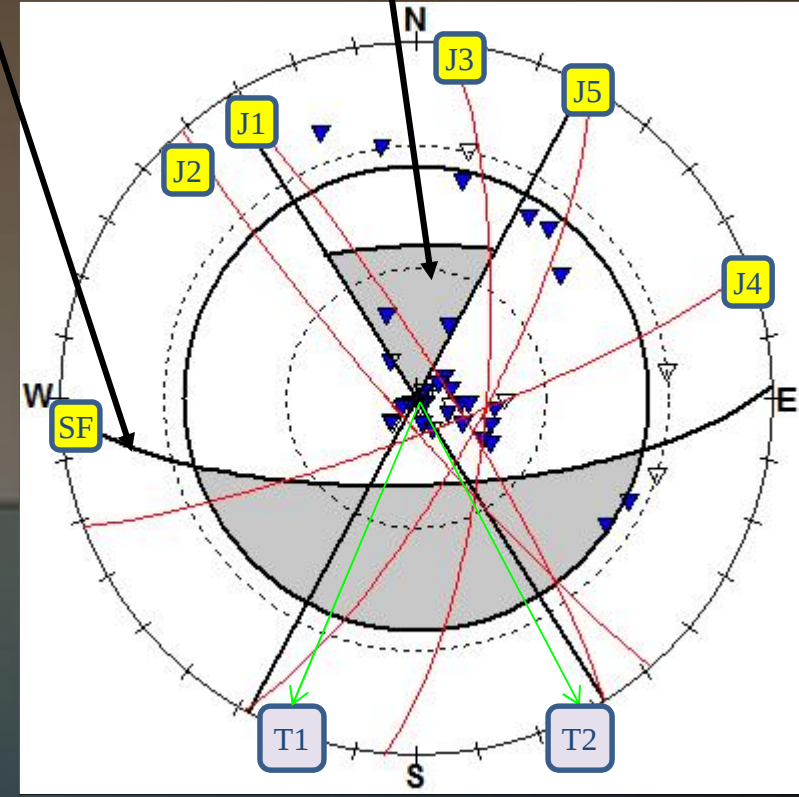
STEREONET PLOT FOR OUTROP 3

(LENGTH: 24M)

Orientation of Slope Face:
70/178



Plane/Wedge Failure Zone



Toppling Failure Zone

INTERPRETATION:

1. POTENTIAL WEDGE FAILURES

- I. W1: INTERSECT BETWEEN J1 & J2
- II. W2: INTERSECT BETWEEN J2 & J3

2. POTENTIAL TOPPLE FAILURES

Set of joints:

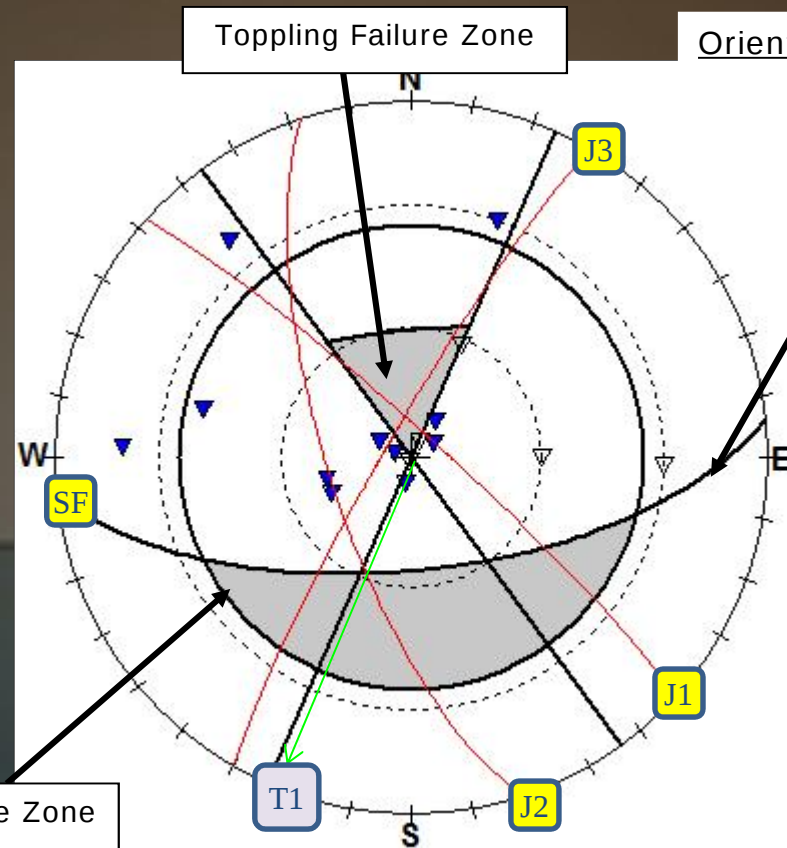
- (J1) 83/058
- (J2) 85/229
- (J3) 74/095
- (J4) 82/159
- (J5) 75/119

LEGEND:

▲ Joint/shear plane

STEREONET PLOT FOR OUTROP 4 (LENGTH: 50M)

INTERVAL: 20 – 50M



Orientation of Slope Face:
64/174

Set of joints:
(J1) – 84/042
(J2) – 71/252
(J3) – 84/300

Plane/Wedge Failure Zone

LEGEND:

▲ Joint/shear plane

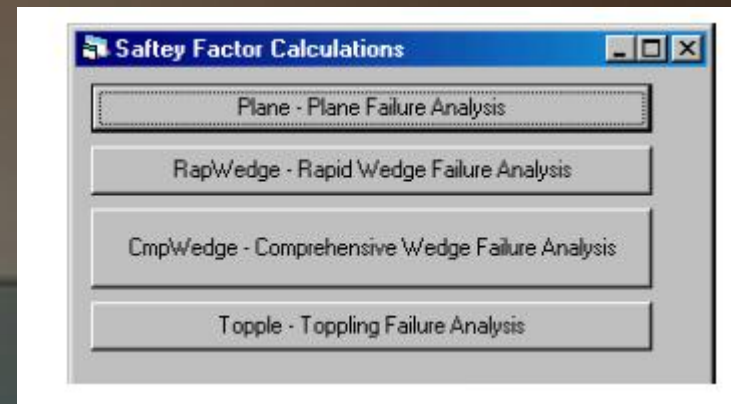
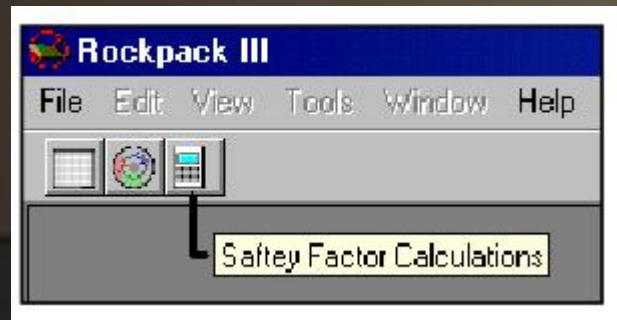
INTERPRETATION:

1. NO POTENTIAL WEDGE FAILURE
2. POTENTIAL TOPPLE FAILURE

B.SAFETY FACTOR ANALYSIS

**OVERVIEW of SAFETY FACTOR CALCULATION PROGRAMS
(*PLANE, RAPWEDGE, CMPWEDGE and TOPPLE*)**

Starting the safety factor programs



Plane Failure Analysis Input Screen

Plane Failure Analysis Input Screen

File View Units Reset Help

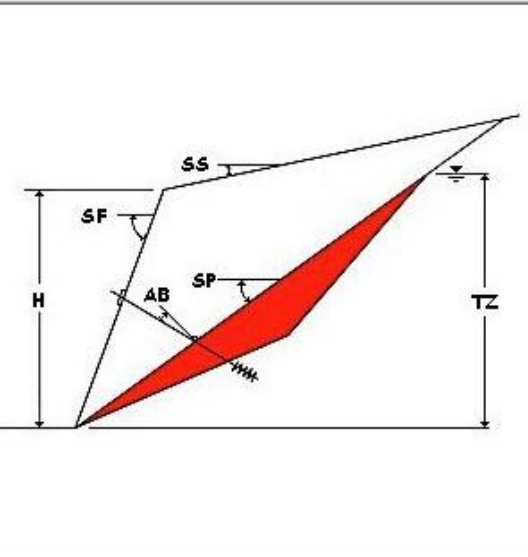


Diagram illustrating the geometry of a slope failure analysis. The failure plane is labeled AB, and the failure surface is labeled SP. The height of the slope is H, the failure plane inclination is SF, the failure surface inclination is SS, the failure plane angle is AB, the failure surface angle is SP, and the tension crack is labeled TZ.

Status: READY

CALCULATE FACTOR OF SAFETY

Optional Input Data

(AC) Horizontal Acceleration G

(TZ) Amount of Discontinuity decimal%

(VSUR) Vertical Surcharge lb(f)

(HSUR) Horizontal Surcharge lb(f)

Standard Input Data

(H) Height ft

(SF) Inclination of Slope Face °

(SS) Inclination of Upper Slope °

(SP) Inclination of Failure Plane °

(CO) Cohesive Strength of Failure Surface lb(f)

(PH) Friction Angle of Failure Surface °

(GR) Density of Rock lb(f)/ft³

(GW) Density of Water lb(f)/ft³

Bolt Data

(AB) Starting Rock Bolt Angle °

(AR) Ending Rock Bolt Angle °

(AA) Bolt Angle Increment °

(T1) Starting Bolt Tension lb(f)

(T2) Ending Bolt Tension lb(f)

(T3) Bolt Tension Increment lb(f)

Tension Crack Data

☒ No Tension Crack

☐ Tension Crack Location Known

☐ Tension Crack Location Unknown

(B) Horizontal Distance of Tension Crack from Crest ft

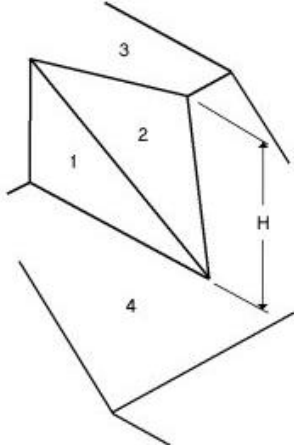
(DZ) Relative Height of Water in Tension Crack decimal%

☒ Dry Slope ☐ Tension Crack filled with Water

Rapid Wedge Failure Analysis Input Screen

Rapid Wedge Failure Analysis

File View Units Reset Help



CALCULATE FACTOR OF SAFETY

Preliminary Data

(GR) Density of Rock lb(f)/ft³

(H) Height of Crest Above Intersection ft

Slope Undercutting

Does the slope face overhang the toe of the slope?

☐ Yes ☒ No

Orientation Data for Planes (1-4)

Plane 1: (D1) Dip Value ° (E1) Dip Direction °

Plane 2: (D2) Dip Value ° (E2) Dip Direction °

Plane 3: (D3) Dip Value ° (E3) Dip Direction °

Plane 4: (D4) Dip Value ° (E4) Dip Direction °

Cohesion and Friction Data

Plane 1: (C1) Cohesion lb(f)/ft² (P1) Friction Angle °

Plane 2: (C2) Cohesion lb(f)/ft² (P2) Friction Angle °

Water Pressure Data

☒ Dry Slope

☐ Cracks Completely Filled (Free Draining)

☐ Alternate Overall Wedge Height Method

(GW) Density of Water lb(f)/ft³

(HW) Overall Vertical Height of Wedge ft

(INTTRND) Trend of line of intersection

(INTPLNG) Plunge of line of intersection

☐ Other

(U1) Pressure on Plane 1 lb(f)/ft²

(U2) Pressure on Plane 2 lb(f)/ft²

Comprehensive Wedge Failure Analysis Input Screen

Comprehensive Wedge Failure Analysis

File View Units Reset Help

Wedge Data

☐ Include Tension Crack

(TL) Distance of Tension Crack from Crest along Plane 1 Trace ft

☐ Include Cable or Bolt Tension

(T) Cable or Bolt Tension lb(f)
(DT) Plunge of Tensile Force °
(ET) Trend of Tensile Force °

☐ Include External Load

(E) External Load lb(f)
(DE) Plunge of External Force °
(EE) Trend of External Force °

Cohesion and Friction Data

Plane 1: (C1) Cohesion lb(f)/ft²
(P1) Friction Angle °

Plane 2: (C2) Cohesion lb(f)/ft²
(P2) Friction Angle °

Water Pressure

☒ Dry Slope
☐ Cracks Completely Filled (Free Draining)
☐ Other

(GW) Density of Water lb(f)/ft³
(U1) Pressure on Plane 1 lb(f)/ft²
(U2) Pressure on Plane 2 lb(f)/ft²
(U5) Pressure on Plane 5 lb(f)/ft²

Orientation Data

Plane 1: (D1) Dip Value ° (E1) Dip Direction °
Plane 2: (D2) Dip Value ° (E2) Dip Direction °
Plane 3: (D3) Dip Value ° (E3) Dip Direction °
Plane 4: (D4) Dip Value ° (E4) Dip Direction °
Plane 5: (D5) Dip Value ° (E5) Dip Direction °

Preliminary Data

(GR) Density of Rock lb(f)/ft³
(H1) Slope Height Referred to Plane 1 ft

Slope Undercut Data

Does the slope face overhang the toe of the slope?
☐ Yes ☒ No

CALCULATE FACTOR OF SAFETY

Toppling Failure Analysis Input Screen

Topple Failure Analysis

File Units Reset Help

Input Data

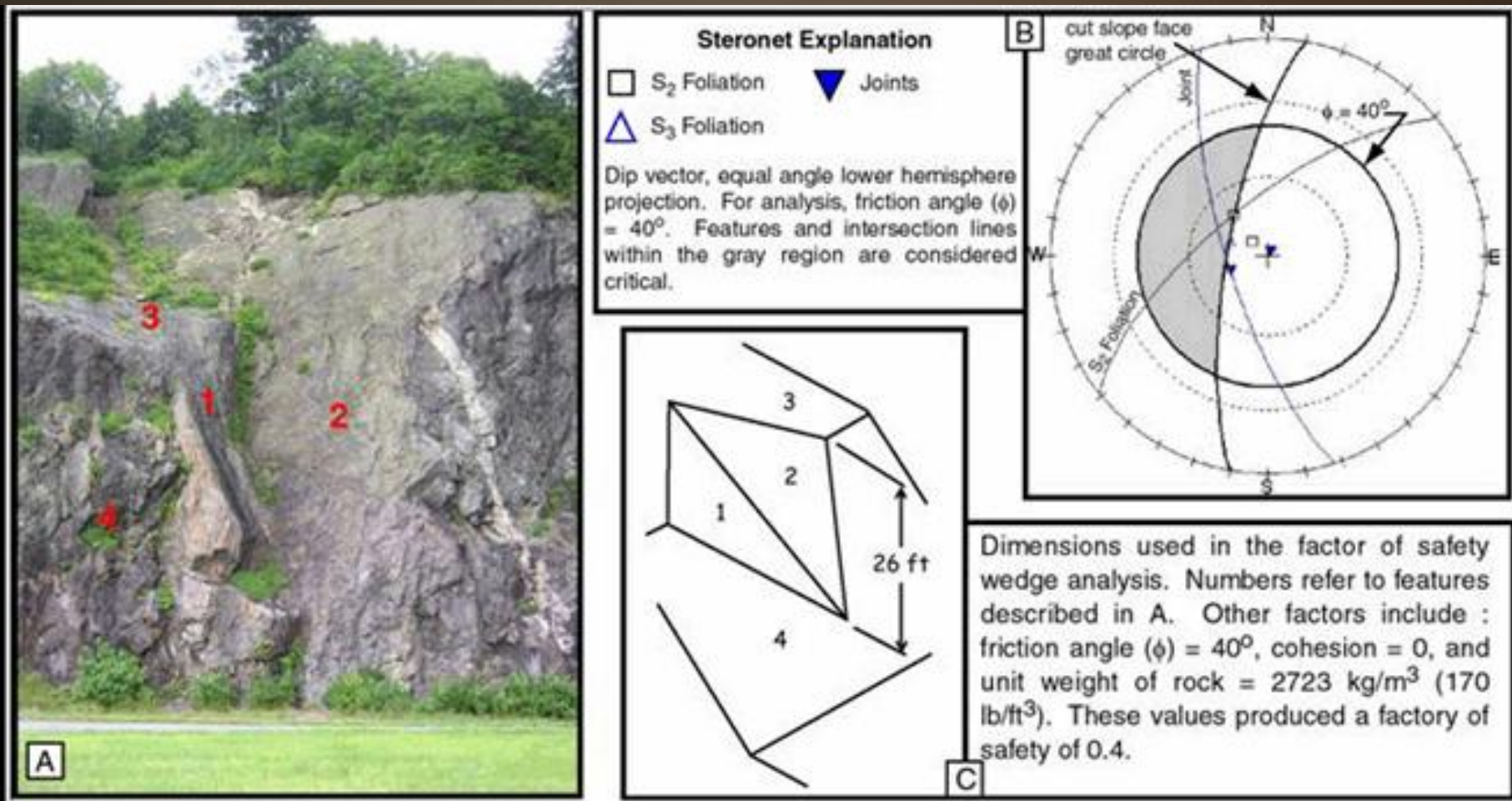
☒ Calculate Sum of Moments
☐ Calculate the tension required to bolt in place
☐ Calculate the thickness required to prevent toppling

Height of the Block ft
Width of the Block ft
Angle of the Block °
Density of Water lb(f)/ft³
Density of Rock lb(f)/ft³
Tension of the Rock Bolt lb(f)
Height of Water in Tension Crack ft
Height of Rock Bolt in Block ft

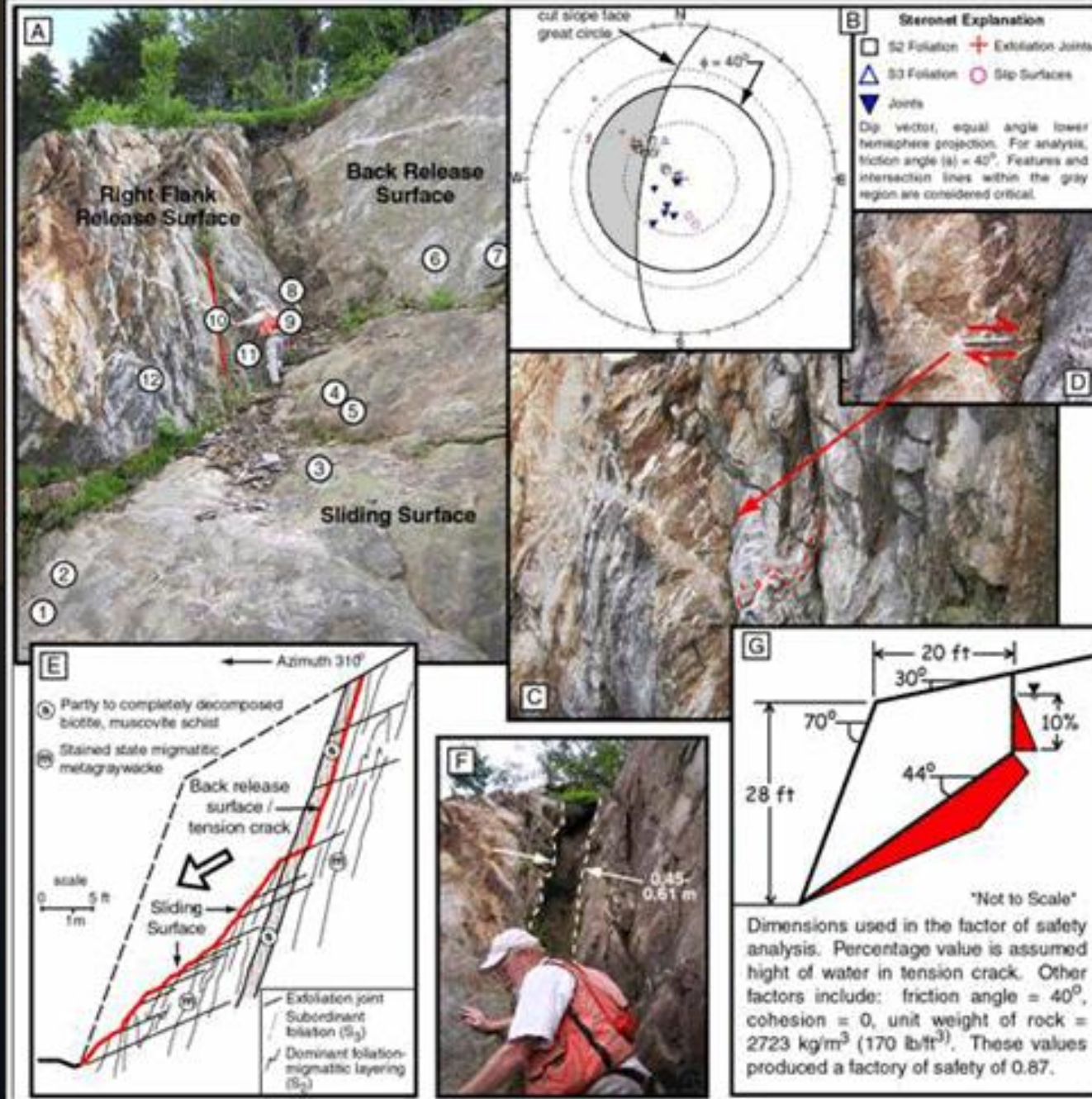
Results

CALCULATE

EXAMPLE OF WEDGE FAILURE ANALYSIS

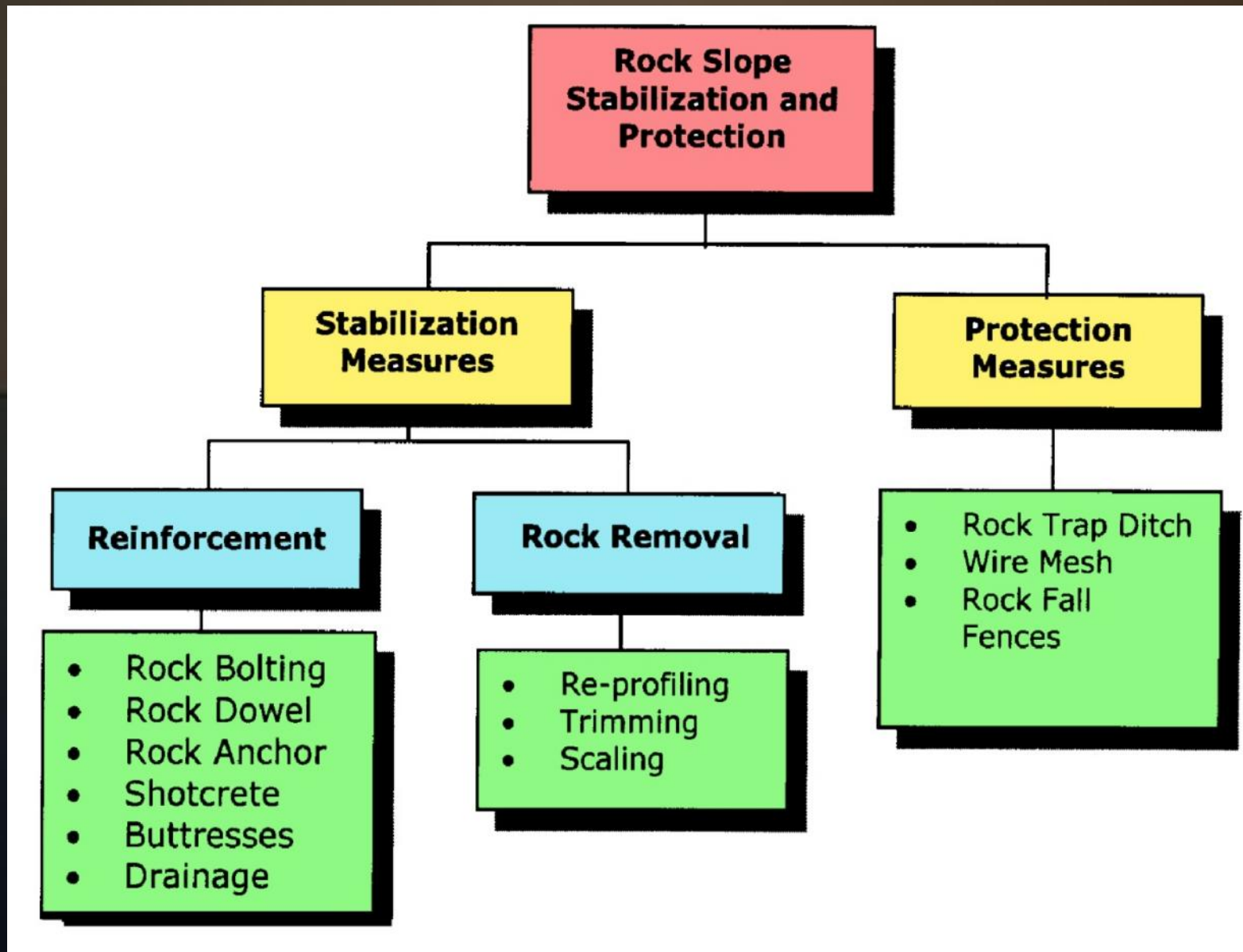


Example of Plane Failure Analysis

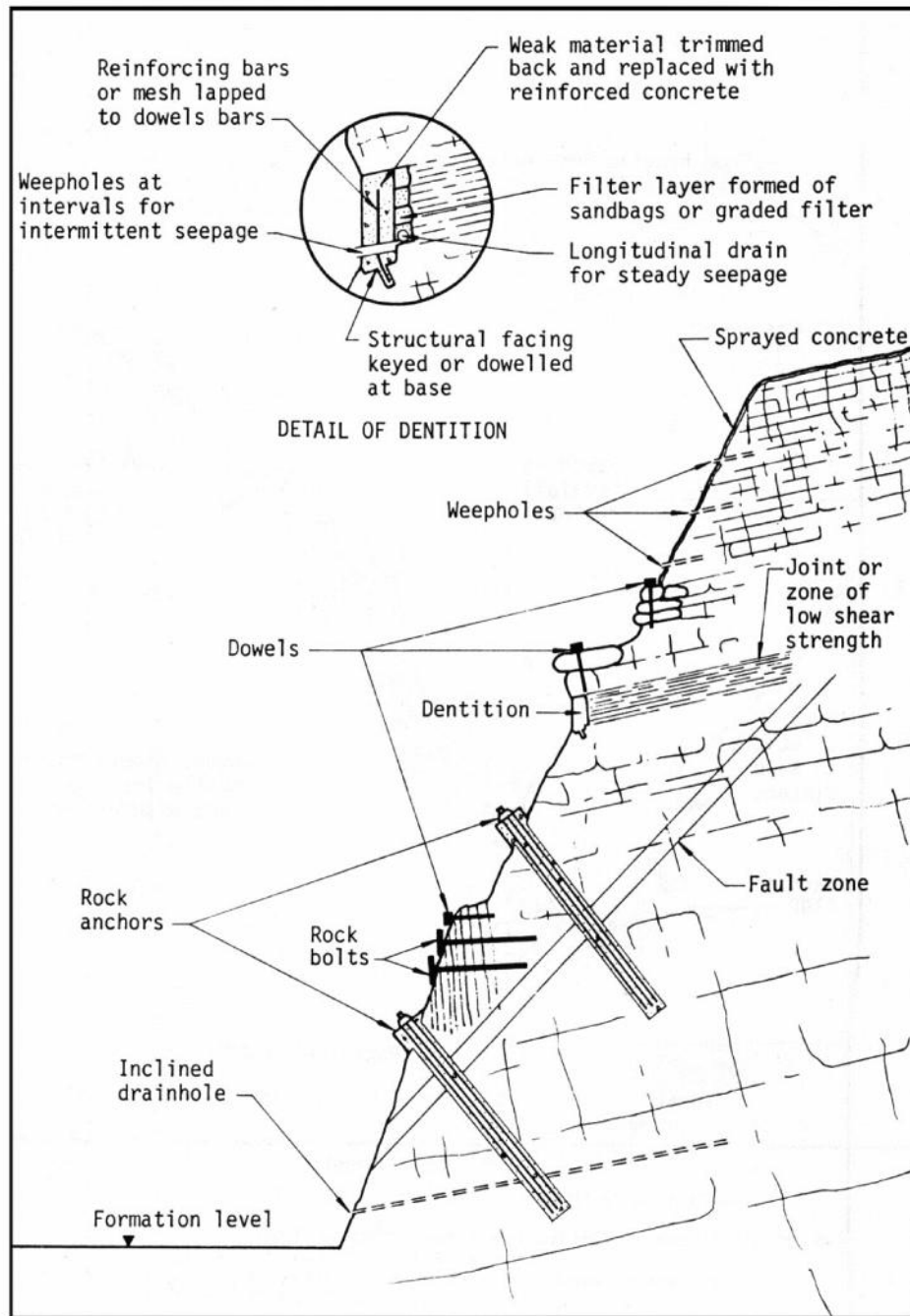


4.0 ROCK SLOPE STABILIZATION AND PROTECTION

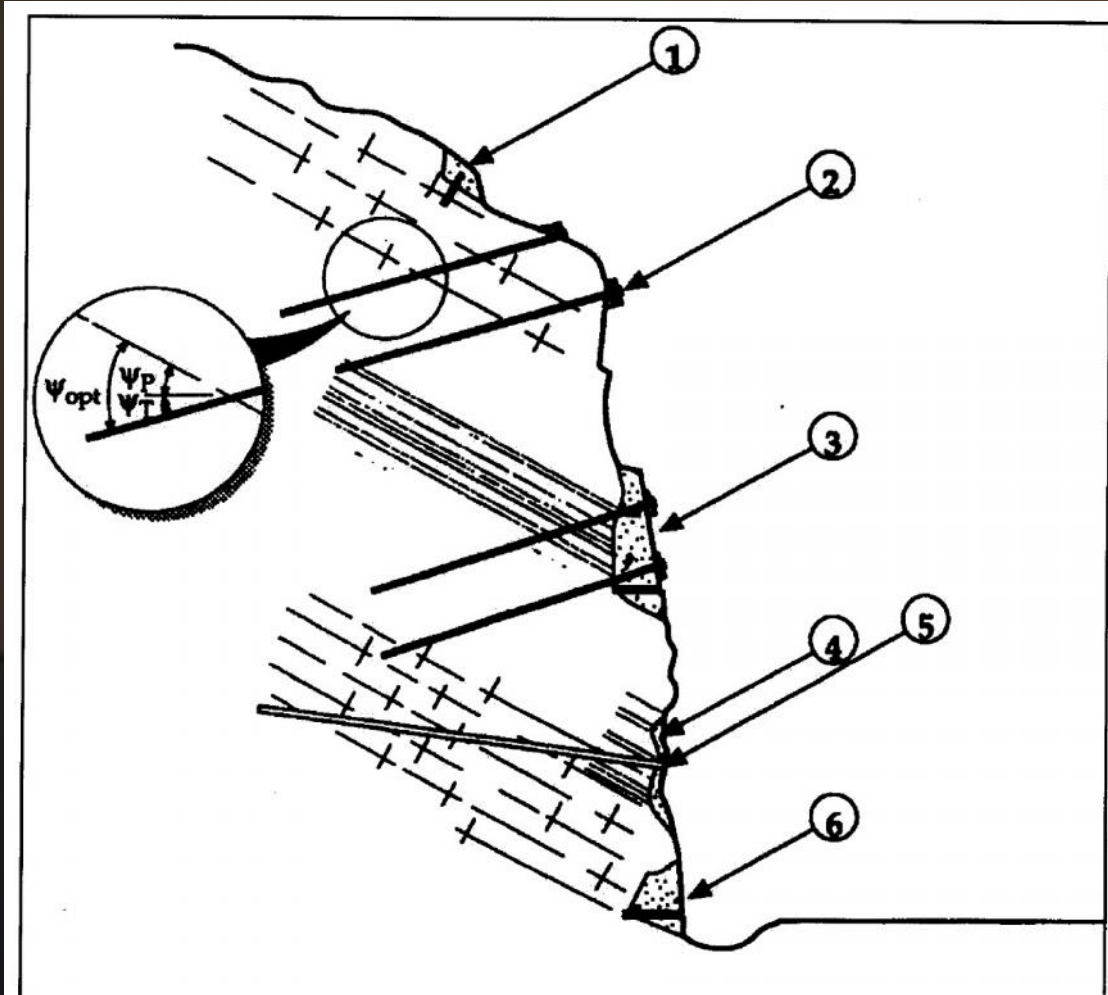
ROCK SLOPE STABILIZATION & PROTECTION MEASURES



VARIOUS METHODS OF STABILISING ROCK SLOPES



- Sprayed Concrete/Shotcrete
- Dowels
- Rock Bolts
- Rock Anchors / Ground Anchors
- Inclined Drainholes / Horizontal Drains
- Dentition

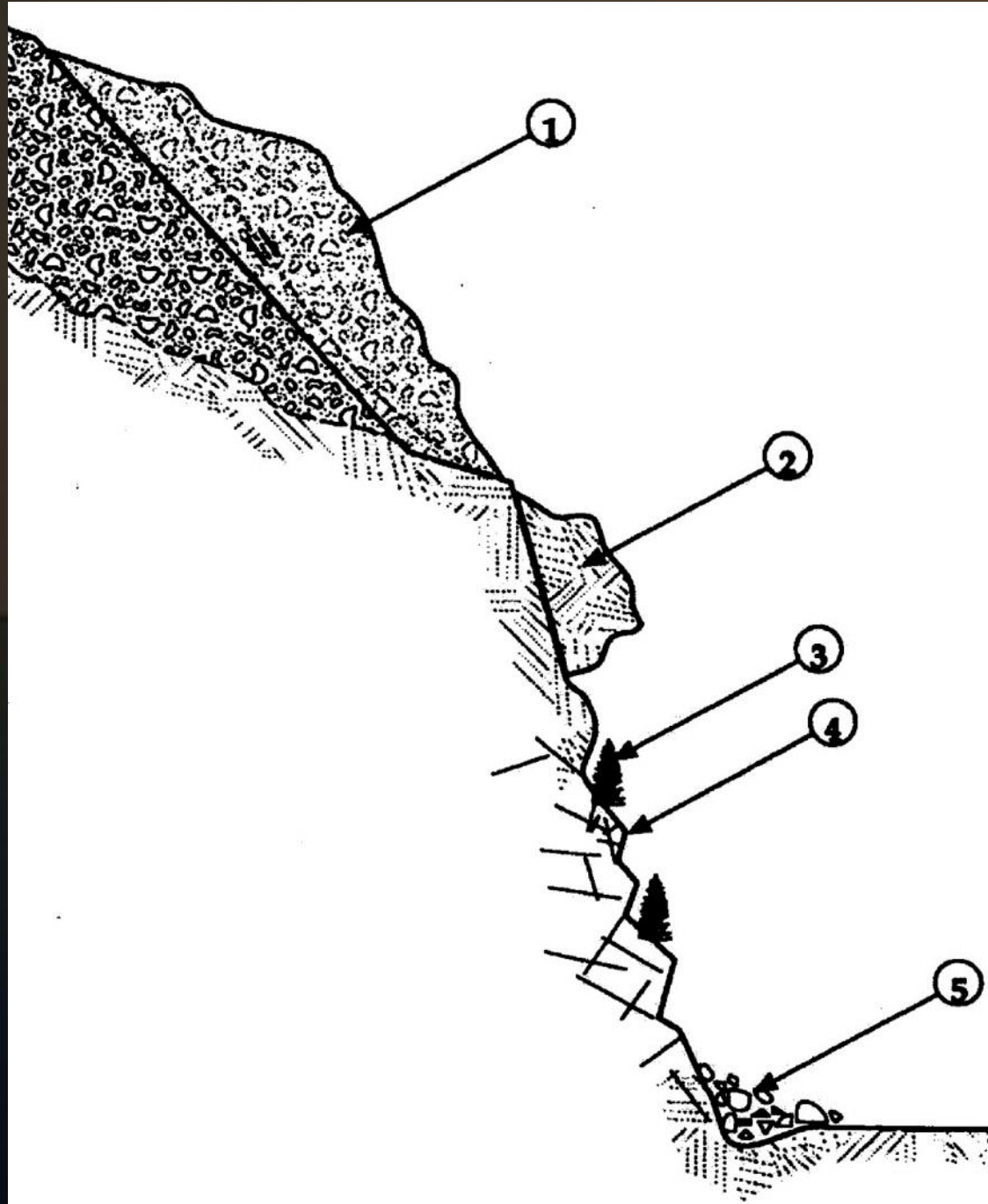


1. Reinforced concrete dowel to prevent loosening of slab at crest.
2. Tensioned rock anchors to secure sliding failure along crest.
3. Tieback wall to prevent sliding failure on fault zone.
4. Shotcrete to prevent ravelling of zone of fractured rock.
5. Drain hole to reduce water pressure within slope.
6. Concrete buttress to support rock above cavity.

REINFORCEMENT METHODS OF STABILISING ROCK SLOPES

(After Keith et al., 1996)

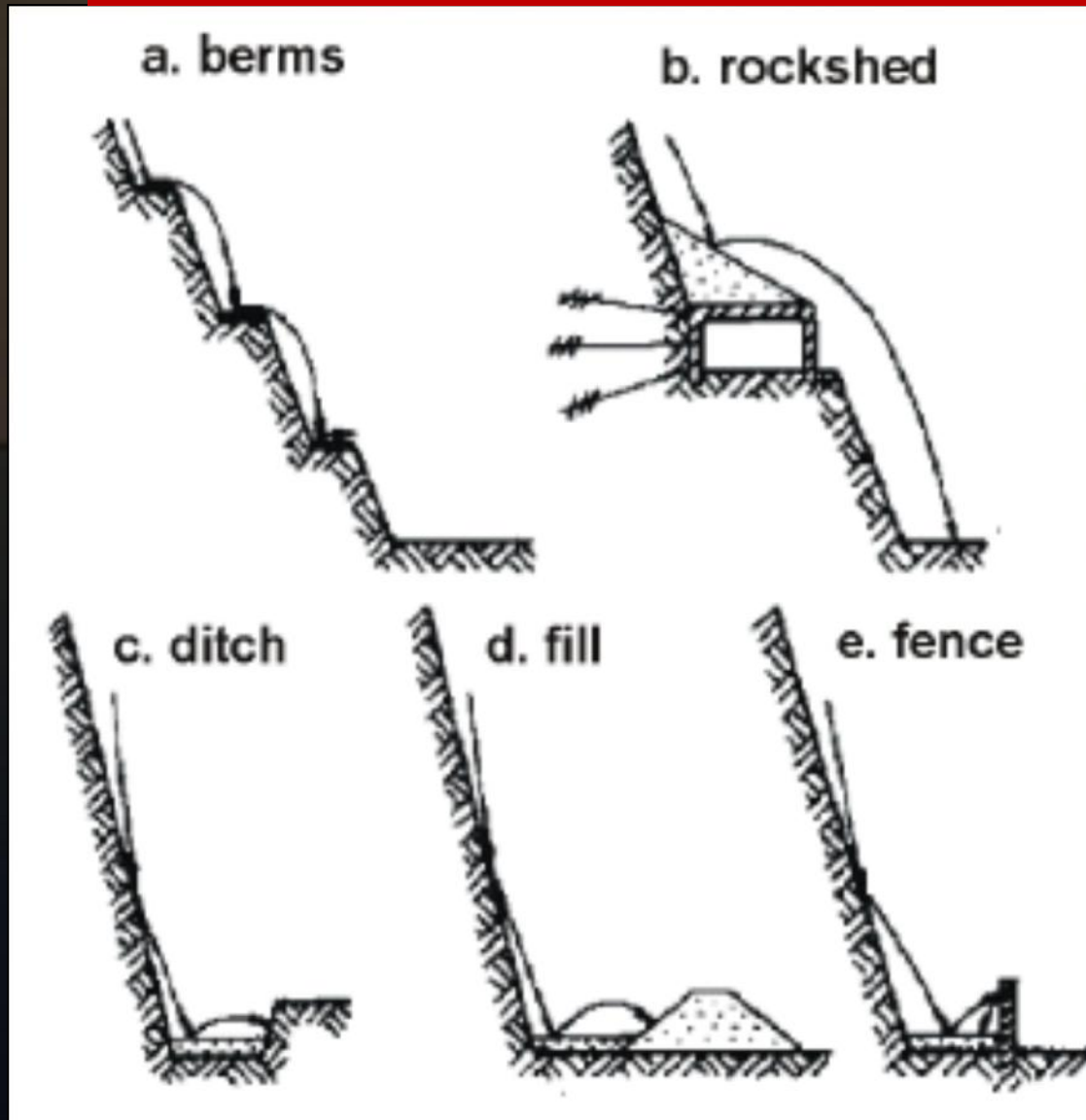
REMOVAL METHODS OF ROCK SLOPES



1. Reprofilng/resloping of unstable weathered material in upper part of slope
2. Removal of rock overhang by trim blasting.
3. Removal of trees with roots growing in cracks
4. Hand scaling of loose blocks in shattered rock
5. Clean ditch or Rock Trap Ditch

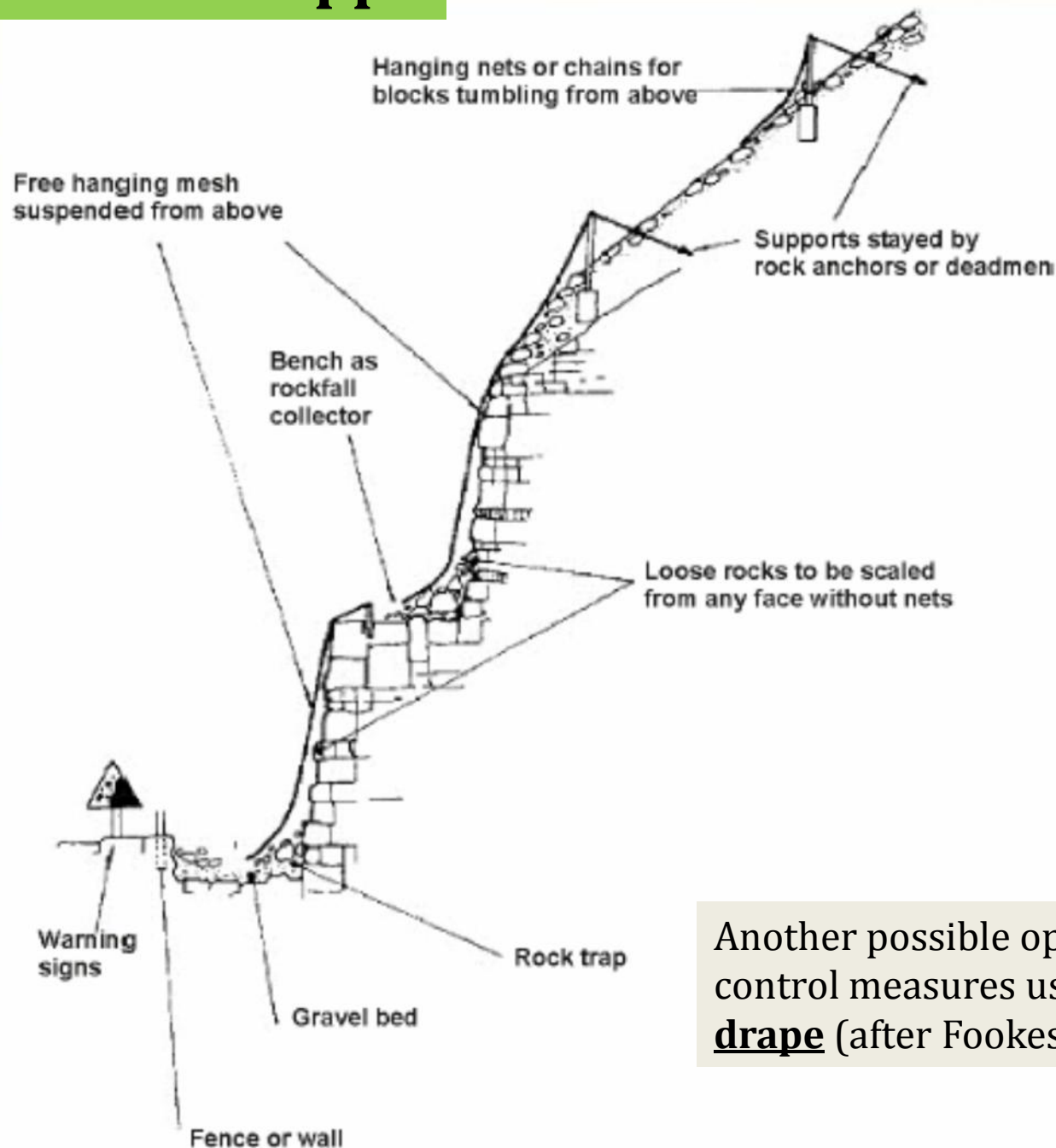
(After Keith et al., 1996)

ROCK SLOPE PROTECTION METHODS



Possible measures to restrain and to reduce damage due to rockfalls (after Spang, 1987)

Wire Mesh Drappe



Another possible option of rockfall control measures using **wire mesh drappe** (after Fookes & Sweeney, 1976)

ROCK SLOPE STABILISATION (REINFORCEMENT)

- Some common practices in Malaysia -

Wire Mesh Drapery

- **Concept:**

- The design intent of a mesh is to cover a slope and control the rockfall trajectory by restricting the rock's movement;
- Mesh weight and strength provides an important stabilizing factor. High tensile strength wire mesh
- Hanging loose on the slope face
- The rockfall drape shall be capable of being pulled on/out at the bottom to clear rocks retained upslope from the draped slope and for rock removal.
- The top of the wire mesh drapery shall be secured to a top support cable.



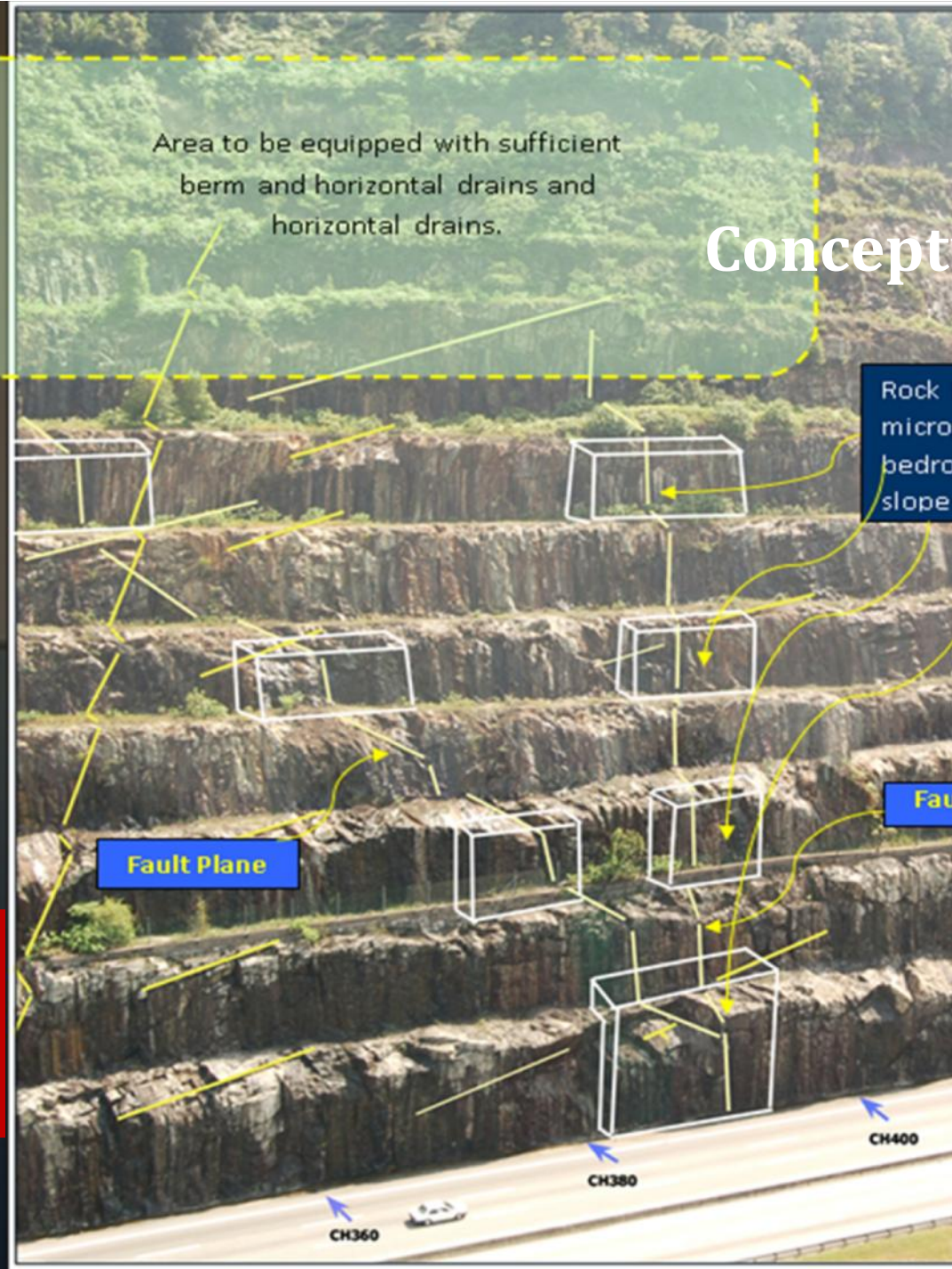
Vegetation



Geocell + soil improvements to support vegetation growth on poor sub-soil conditions

- Proposed rock buttresses to arrest a large scale wedge

ROCK BUTRESS



Conceptual Design

Rock Buttress – founded on micropiles socketed in sound bedrock, and tied back to the slope with rock anchors.

Fault Plane

Fault Plane

CH360

CH380

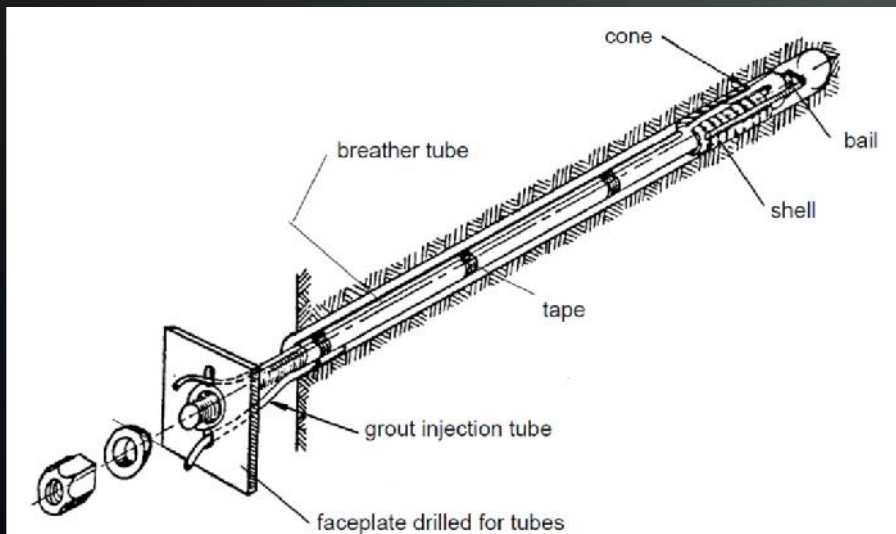
CH400

IMPLEMENTATION



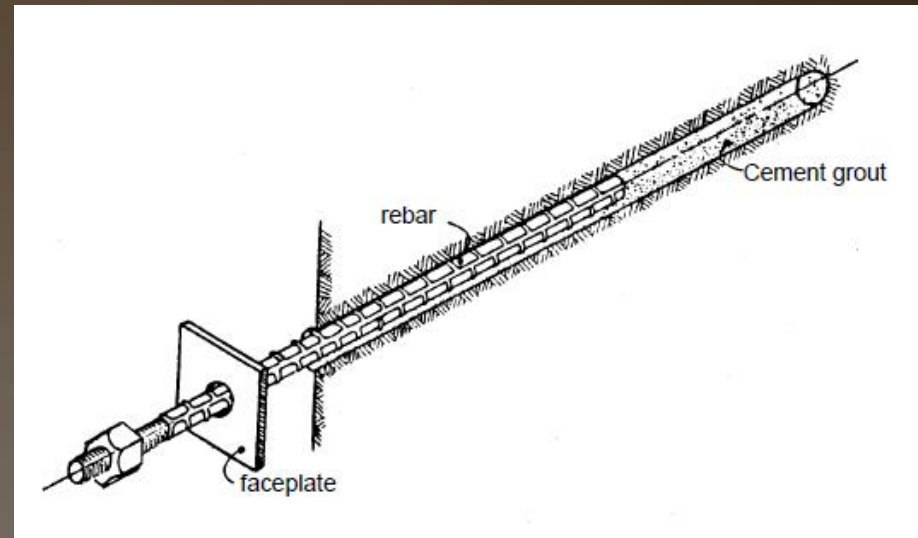
ROCKBOLTS

- Rockbolts generally consist of plain steel rods with a mechanical anchor at one end and a face plate and nut at the other.
- They are always tensioned after installation.
- the space between the bolt and the rock can be filled with cement or resin grout.



DOWELS

- Dowels or anchor bars generally consist of deformed steel bars which are grouted into the rock.
- No tensioning!
- In order to be effective, dowels have to be installed before significant movement in the rock mass has taken place.



Grouted dowel using a deformed bar inserted into a grout-filled hole.

ROCK SLOPE STABILISATION

(ROCK REMOVAL)

- Some common practices in Malaysia -

Mechanized Rock Removal/Scaling

Rock Scaling



Rock Trimming



Rock Scaling

ROCK FALL PROTECTION MEASURES

A) BERMS/BENCHING



Lebuhraya Ipoh-Changkat Jering, PLUS.

CATCH BERM



CATCH BERM



A 20m wide catch berm
on Bukit Lanjan Rock Slope

..and it was installed with Geobrugg Rock Fall
Fence (after Euroculture Sdn. Bhd., 2005)



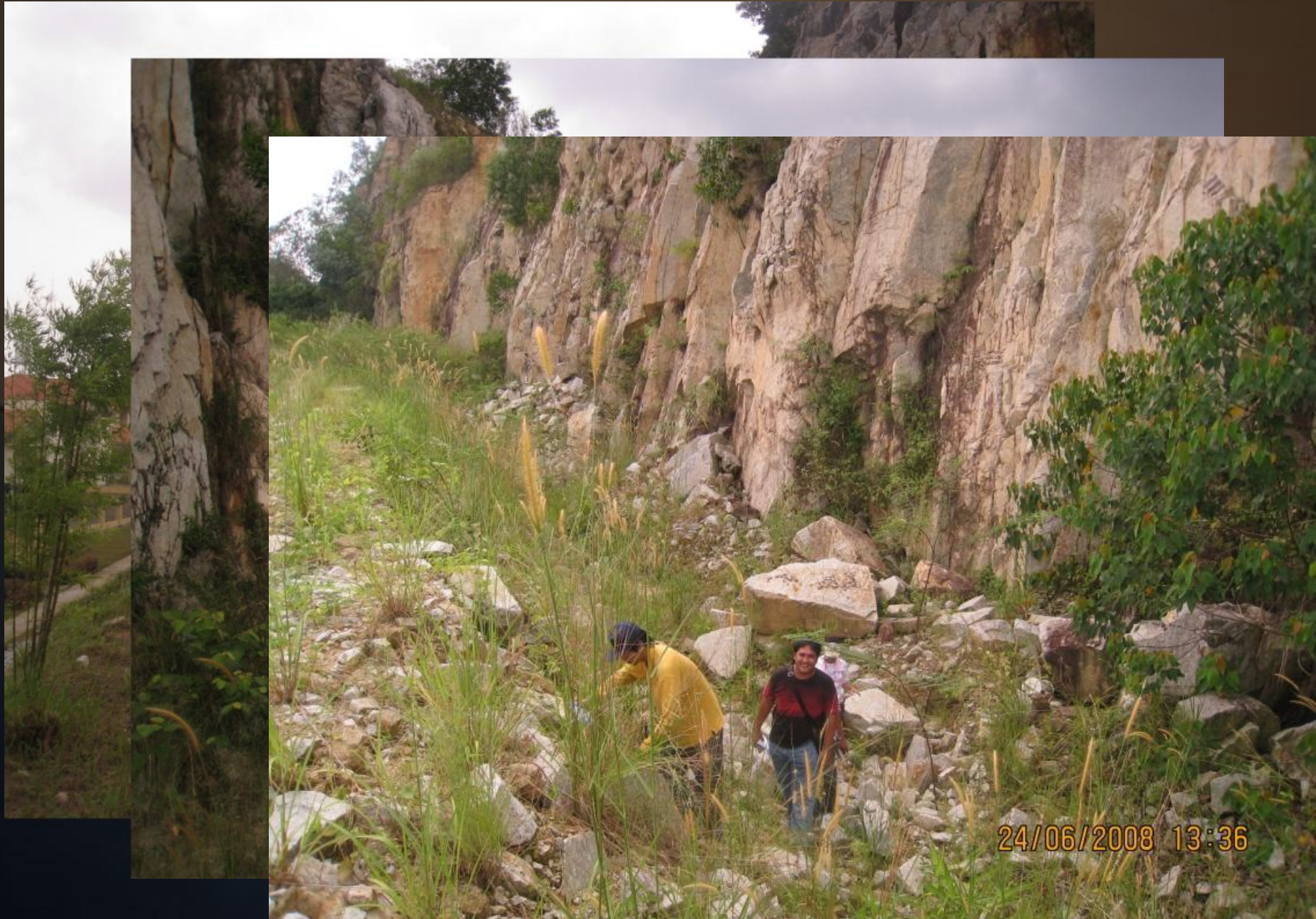
*Photo 2 : Completed Geobrugg Rock Fall
Fence at 20m Wide Berm*

B) ROCKSHED



Some examples from Switzerland

C) ROCK TRAP DITCH & FILL – RAWANG PERDANA



D) ROCK FALL FENCE/BARRIER



Geobrugg ring net shown restraining a boulder. These nets can be designed with energy absorbing capacities of up to 2500 kNm which is equivalent to a 6 tone boulder moving at 20 m per second.

D) ROCK FALL FENCE/BARRIER



Geobruigg energy absorbing ring. When subjected to impact loading the ring deforms plastically and absorbs the energy of the boulder.

D) ROCK FALL FENCE

Application of Geobrugg Rockfall Barrier – the Malaysian Experience



- An example rockfall fence / barrier installed at:
 - Bukit Lanjan Rock Slope, NKVE Highway.
 - Ipoh-Chkt Jering Highway

D) ROCK FALL BARRIER



Tom Price Iron Ore Mine, West Australia (after Geobruigg, 2006)

EXAMPLE: ROCK FALL FENCE @ GUNUNG PASS



E) WIRE MESH DRAPPE



Top edge with hinged supports and suspension of the TECCO® drappe



TECCO® drappe in June 2005 after installation; Ramsau Germany (after Geobrugg AG)

