

# ***FORENSIC SITE INVESTIGATION & INSTRUMENTATION***



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# SESSION 1: SITE INVESTIGATION & INSTRUMENTATION





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graph TD; A[SITE INVESTIGATION] --> B[SOIL INVESTIGATION]; A --> C[SURFACE INVESTIGATION];
```

**SITE INVESTIGATION**

**SOIL INVESTIGATION**

**SURFACE INVESTIGATION**

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# SOIL INVESTIGATION



## *OBJECTIVES*

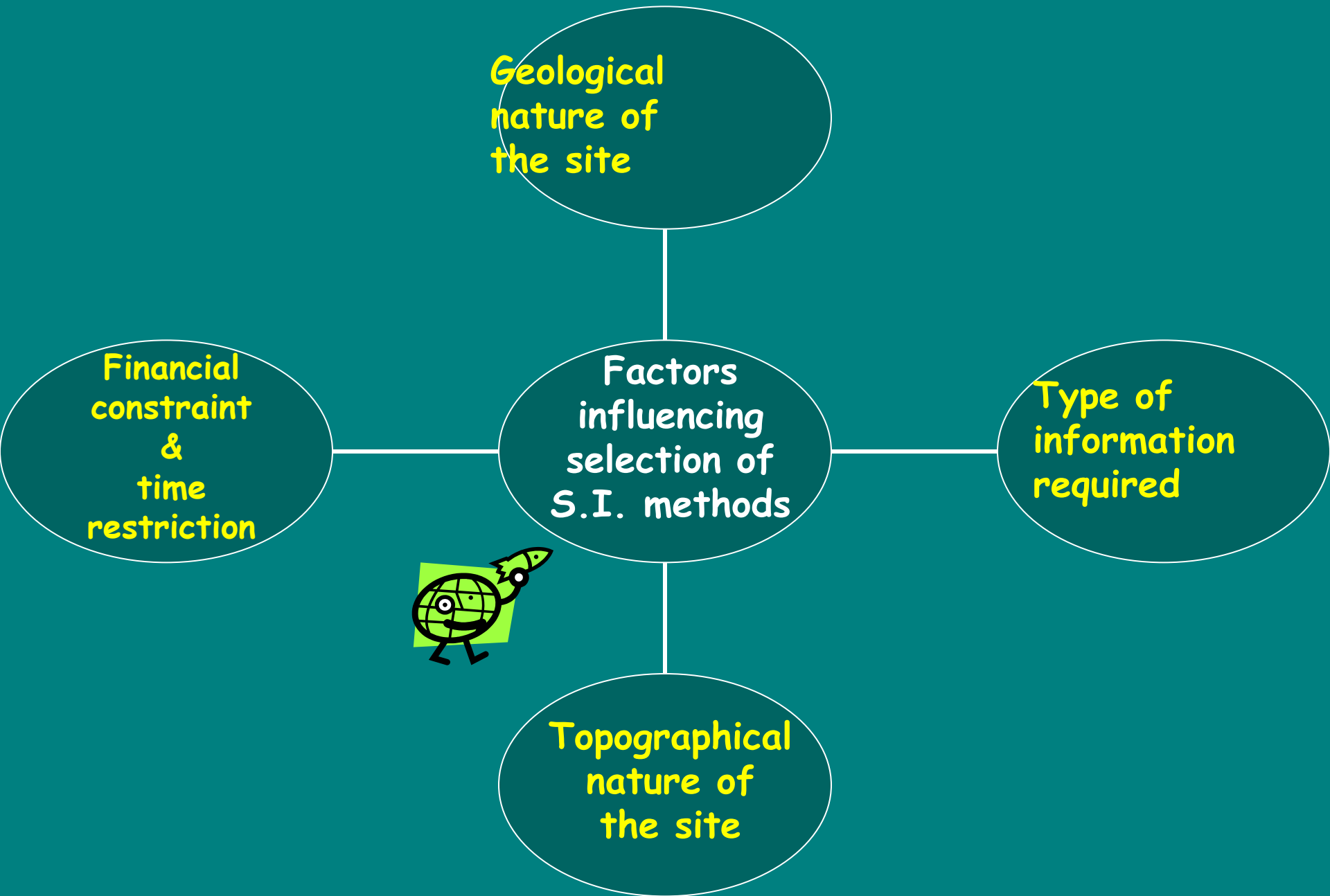
- To study the general suitability of the site for a construction method or an engineering project (feasibility studies)
- To enable a safe, practical and economical design to be prepared.
- To determine the possible difficulties may be encountered by a specific construction method.
- To study the suitability of construction material (soil or rocks).

# STANDARD PENETRATION TEST (SPT)



## OBJECTIVES

- The most economical means of obtaining subsurface information
- A split barrel sampler 450 mm long is driven by a free falling 65 kg hammer.
- The first 150 mm is called the seating drive and not counted.
- The total cumulative no. of blow counts for the last 300 mm is recorded as N-value



# SOIL INVESTIGATION





# SOIL INVESTIGATION





# SPT HAMMER



# SPLIT BARREL SAMPLER



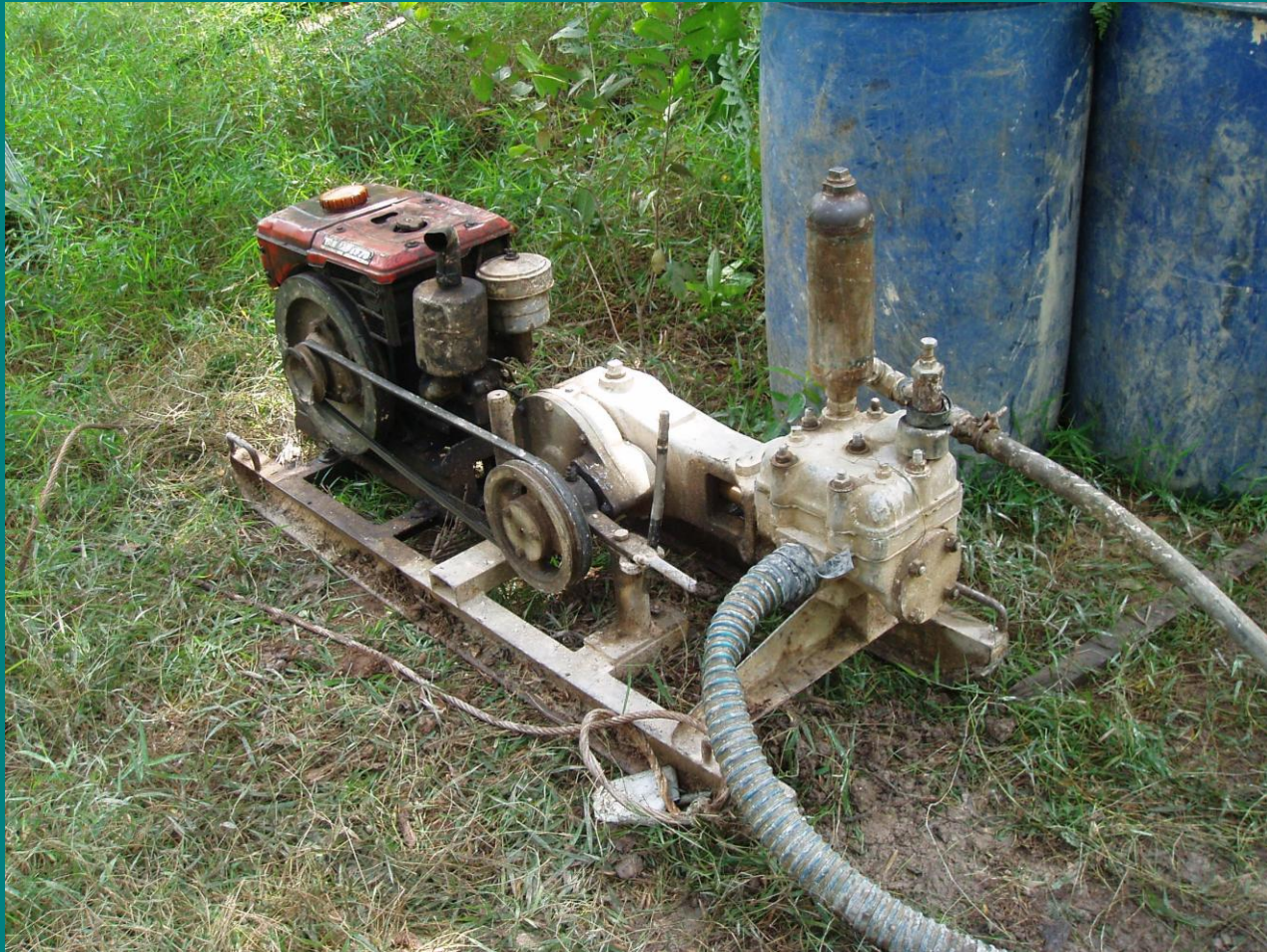


# CPT CONE





# WATER PUMP





# CASING SHOE



# DRILL BIT





# CPT MACHINE



# CPT LOGGING



# INCLINOMETER





# SETTLEMENT GAUGE



# SETTLEMENT GAUGE



# Types of Triaxial Tests

Depending on whether drainage is allowed or not during

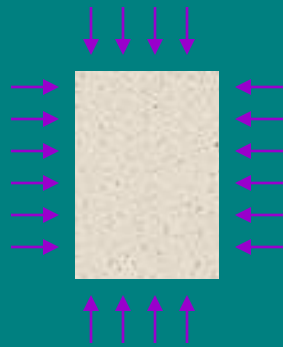
- ❖ initial isotropic cell pressure application, and
- ❖ shearing,

there are three special types of triaxial tests that have practical significances. They are:

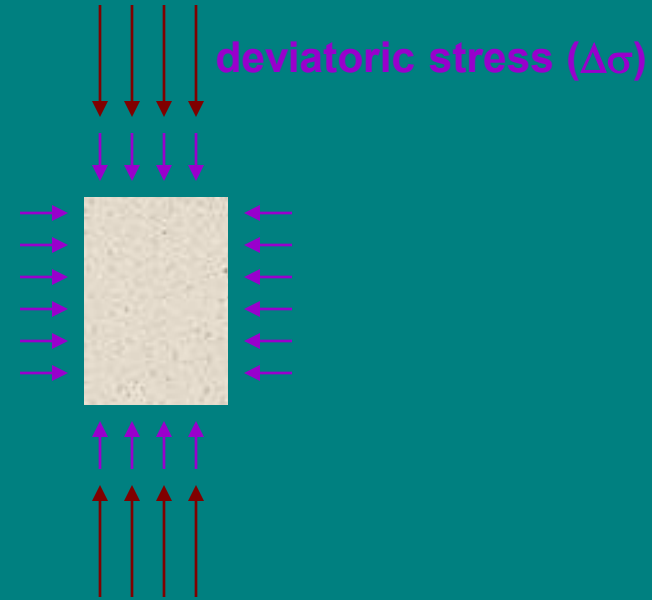
**Consolidated Drained (CD) test**  
**Consolidated Undrained (CU) test**  
**Unconsolidated Undrained (UU) test**



# Types of Triaxial Tests



Under all-around  
cell pressure  $\sigma_c$



Shearing (loading)

Is the drainage valve open?

yes

no

**C**onsolidated  
sample

**U**nconsolidated  
sample

Is the drainage valve open?

yes

no

**D**rained  
loading

**U**ndrained  
loading

# PERMEABILITY TEST



# TRIAXIAL COMPRESSION TEST





# EXTRUDING OF SAMPLE



# STORAGE



# SURFACE MARKER





# PIEZOMETER



# MAGNETIC EXTENSOMETER





# INCLINOMETER





# SAMPLING TUBE



# MACKINTOSH PROBE



## *OBJECTIVES*

- Results are used to determine thickness of unsuitable material to be removed and also for preliminary design of embankments.
- Usually carried out near hand auger or deep boring positions and filling areas to verify the consistency of subsoil.

# MACKINTOSH PROBE



The diagram shows a cross-section of the ground with a horizontal line representing the ground surface. Below the surface, there is a rectangular area representing the soil. A vertical line with a downward-pointing arrow indicates the direction of the probe's movement. The arrow is labeled 'MACKINTOSH PROBE'.

- Light dynamic test
- Cone is driven directly into the soil by driving a hammer (5 kg) through a free drop of 280mm.
- The results are recorded as number of blows per 300mm penetration.
- Maximum penetration is 12m or 400blows/300mm; whichever comes first.



# Mackintosh Probe



# LABORATORY TESTS



- Laboratory testing of soil and rock should be as recommended in BS5930:1999.
- Testing should be carried out in accordance with BS1377:1990.

# LABORATORY TESTS



```
graph TD; A[LABORATORY TESTS] --> B[SHEAR STRENGTH]; A --> C[COMPRESSION]
```

## *SHEAR STRENGTH*

- ***Triaxial load cells*** are the most widely used laboratory shear strength test for fine soils, allowing control of drainage conditions and porewater measurements.
- The test specimen is obtained from carefully extruded undisturbed tube samples.



# LABORATORY TESTS



## *SHEAR STRENGTH*

### **UNCONSOLIDATED, UNDRAINED**

With no drainage allowed, which gives an initial *in situ* condition.

### **CONSOLIDATED UNDRAINED**

With drainage allowed, until consolidation is complete, then continue undrained with porewater pressure measurements.

### **DRAINED**

With drainage allowed throughout the consolidation and shearing stages.

# LABORATORY TESTS

```
graph TD; A[LABORATORY TESTS] --> B[UNDRAINED STRENGTH]; A --> C[DRAINED STRENGTH]; B --> D[SHEAR STRENGTH]; C --> D;
```

## *SHEAR STRENGTH*

### UNDRAINED STRENGTH

Given in total stress terms ( $c_u$  and  $\phi_u$ ).

### DRAINED STRENGTH

Given in effective strength terms ( $c'$  and  $\phi'$ ).

# LABORATORY TESTS



## CONSOLIDATION

### LABORATORY OEDOMETER TEST – (ONE DIMENSIONAL LOADING)

Used to determine the coefficient of volume compressibility ( $m_v$ ) of clays and silts, from which the coefficient of consolidation ( $C_v$ ) is obtained.

These two coefficients enable the magnitude and rate of consolidation under full structural applied load to be estimated.



# SEIVE ANALYSIS TESTS

## OBJECTIVE

To determine grain sizes distribution.

*How?* 

Sieving are performed by arranging various sieves one over the other in the order of their mesh openings, the largest aperture sieve being kept at the top and the smallest aperture sieve at the bottom.

Distributions of gravel and sand particles are determined by sieve analysis; meanwhile clay and silt are determined by sedimentation or wet analysis.

# LABORATORY TESTS



```
graph TD; A[LABORATORY TESTS] --> B[ATTERBERG LIMIT TESTS]; B --> C[LIQUID LIMIT TESTS]; B --> D[PLASTIC LIMIT TESTS]; B --> E[SHRINKAGE LIMIT TESTS];
```

A hierarchical flowchart on a teal background. At the top is a dark teal rectangle with the text 'LABORATORY TESTS' in yellow. A white line descends from this rectangle, branching into three white lines that lead to three rounded rectangular boxes below. The middle box is labeled 'ATTERBERG LIMIT TESTS' in yellow. From the bottom of this middle box, a white line descends to a horizontal white line. From this horizontal line, three white lines branch out to three more rounded rectangular boxes at the bottom, labeled 'LIQUID LIMIT TESTS', 'PLASTIC LIMIT TESTS', and 'SHRINKAGE LIMIT TESTS' in yellow.

ATTERBERG  
LIMIT  
TESTS

LIQUID  
LIMIT  
TESTS

PLASTIC  
LIMIT  
TESTS

SHRINKAGE  
LIMIT  
TESTS

# LIQUID LIMIT TESTS

## *OBJECTIVE*

To determine liquid limit soil in percentage.

### *Liquid Limit?*

Moisture content at point of transition from plastic to liquid state.

## LIQUID LIMIT TESTS

CASAGRANDE METHOD

CONE PENETRATION METHOD



# PLASTIC LIMIT TESTS



## ***OBJECTIVE***

To determine plastic limit soil in percentage.

## ***Plastic Limit?***

Moisture content at point of transition from semisolid to plastic state.

When the soil is rolled into threads of 3.2mm diameter it start to crumbles.

# SHRINKAGE LIMIT TESTS



## ***OBJECTIVE***

To determine shrinkage limit soil in percentage.

## ***Shrinkage Limit?***

Moisture content at which volume of soil mass ceases to change.

# SPECIFIC GRAVITY TESTS



## **OBJECTIVE**

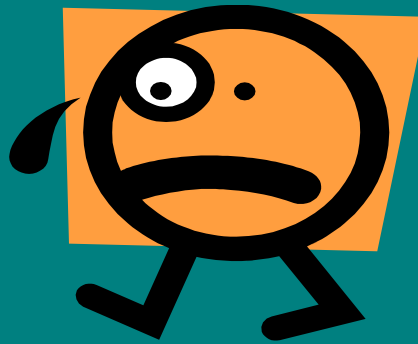
To determine specific gravity of soil.

### ***Specific gravity?***

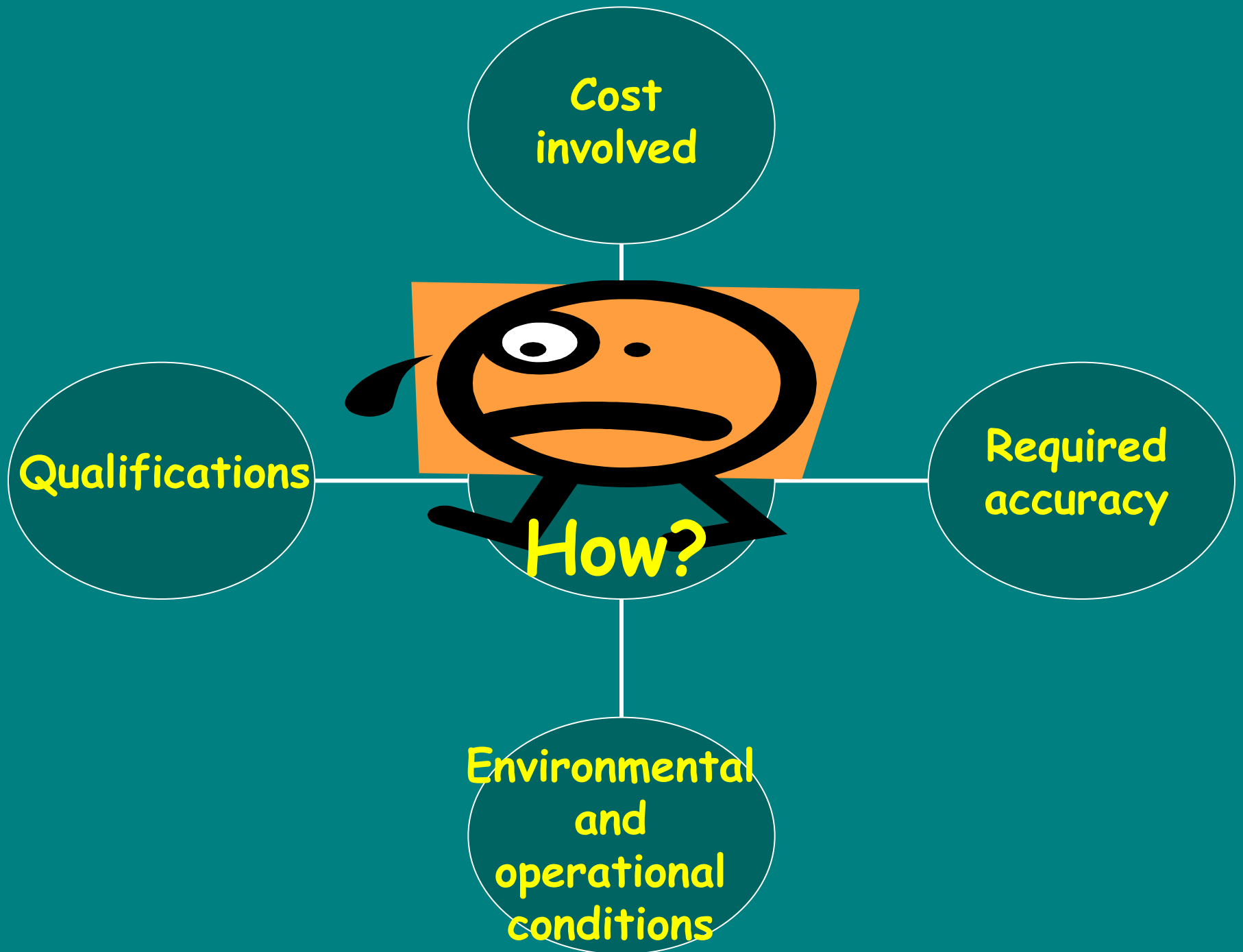
Ratio of weight of certain volume of soil solids to the weight of an equal volume of distilled water at a constant temperature.



# INSTRUMENTATION



How to choose the proper type of instrument?





# **INSTRUMENTATION**

**Standpipe  
Piezometer**

**Vibrating Wire  
Piezometer**

**Magnetic Extensometer**

**Inclinometer**



# PIEZOMETER

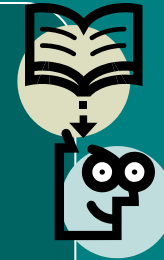


## ***OBJECTIVE***

To monitor water level (static, perched, artesian) in excavations, slopes and dam embankments, to measure excess hydrostatic pressure beneath dams and embankments and to aid in control preloading operations and placement of fill over soft ground.

# PIEZOMETER

*DIVIDED INTO  
2 SYSTEMS*



## OPEN SYSTEM

Measurement are made from the surface and the water level is generally below the surface.

## CLOSED SYSTEM

Measurement are made remotely and the wate level may be at any location.

# STANDPIPE PIEZOMETER

*OPEN SYSTEM*

## APPLICATION

Coarse-grained granular soils, free-draining rock masses.

## ADVANTAGE

Simple, rugged, inexpensive

## DISADVANTAGE

Indicates average head, relatively insensitive, time lag in impervious soils.



# VIBRATING-WIRE PIEZOMETER

## *CLOSED SYSTEM*

### APPLICATION

Fine-grained soils and slow-draining rock masses.

### ADVANTAGE

Extreme sensitive, fast response, continuous recording possible

### DISADVANTAGE

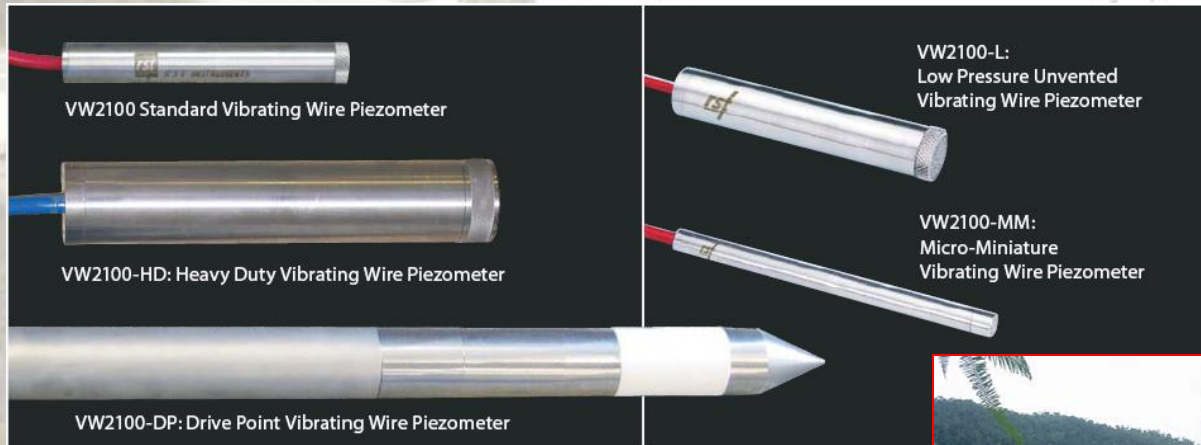
Relatively costly, decreased durability and reliability over other closed systems because of electrical circuitry.

# Real Time Monitoring System

## Vibrating Wire Piezometer (VWP)



# VIBRATING WIRE PIEZOMETER



**Pore Water Pressure/Ground Water  
Table Monitoring.**



# INCLINOMETER

## OBJECTIVE

To measure lateral deflections

## APPLICATIONS

- Installed behind retaining structures or in pile foundations.
- Beneath loaded areas over soft soils
- Monitor slope movements



# INCLINOMETER



## *INSTRUMENTS*

- Contains servo-accelerometers that can detect lateral movements of the order of  $\pm 0.0001$  ft per 2ft of casing.
- Since the voltage output is proportional to the sine of the angle of inclination of the long axis of the sensor, from the vertical, it can be used to measure true deviations from vertical.

# INCLINOMETER

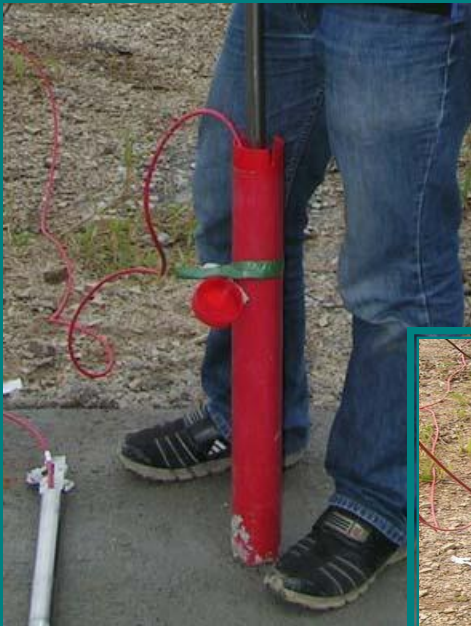


## *INSTALLATION*

- Is lowered and raised in specially grooved casing installed in a borehole and extended to a depth below the anticipated movement zone.
- The casing bottom is often grouted into place to assure fixity.
- For monitoring pile and wall movements, the casing is attached directly to the structural member.

# Real Time Monitoring System

## In-Place Inclinator





# IN-PLACE INCLINOMETER

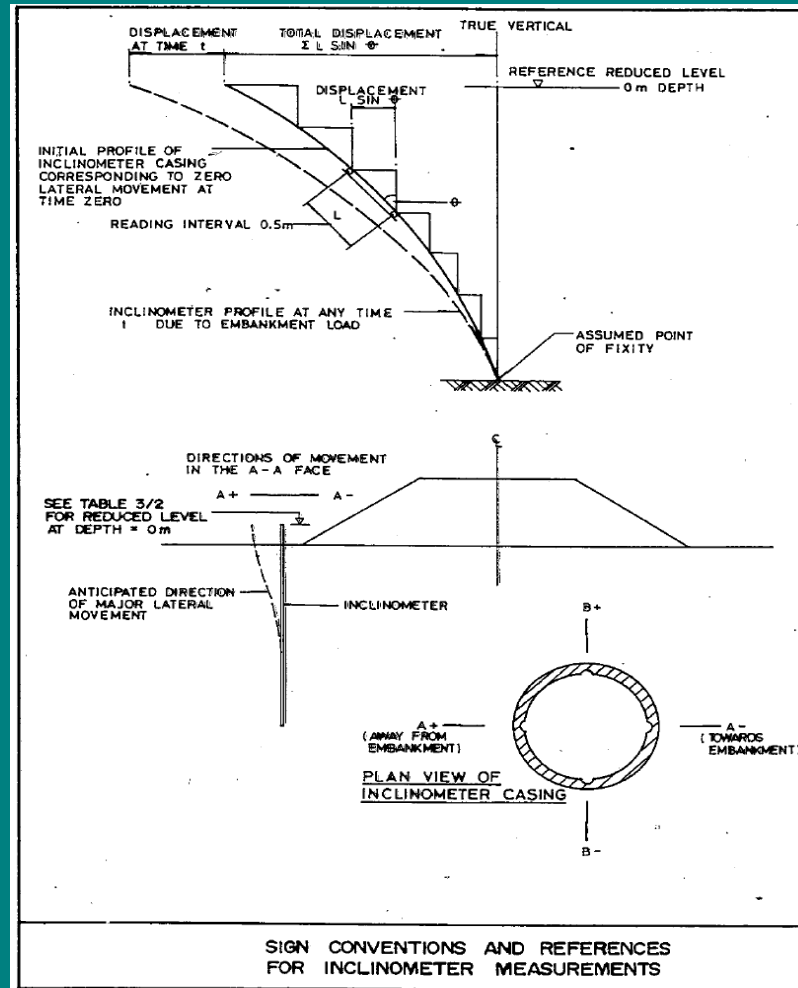
Monitoring Of The  
Lateral Movement.





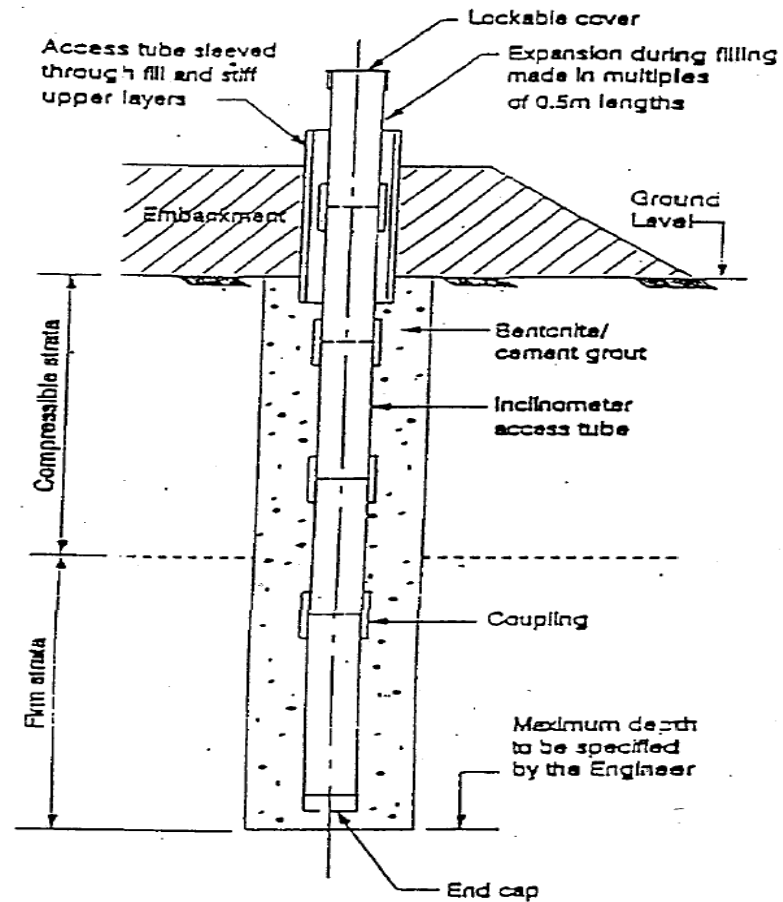
# Instrumentation Details

## Inclinometer- Sign Convention



# Instrumentation Details

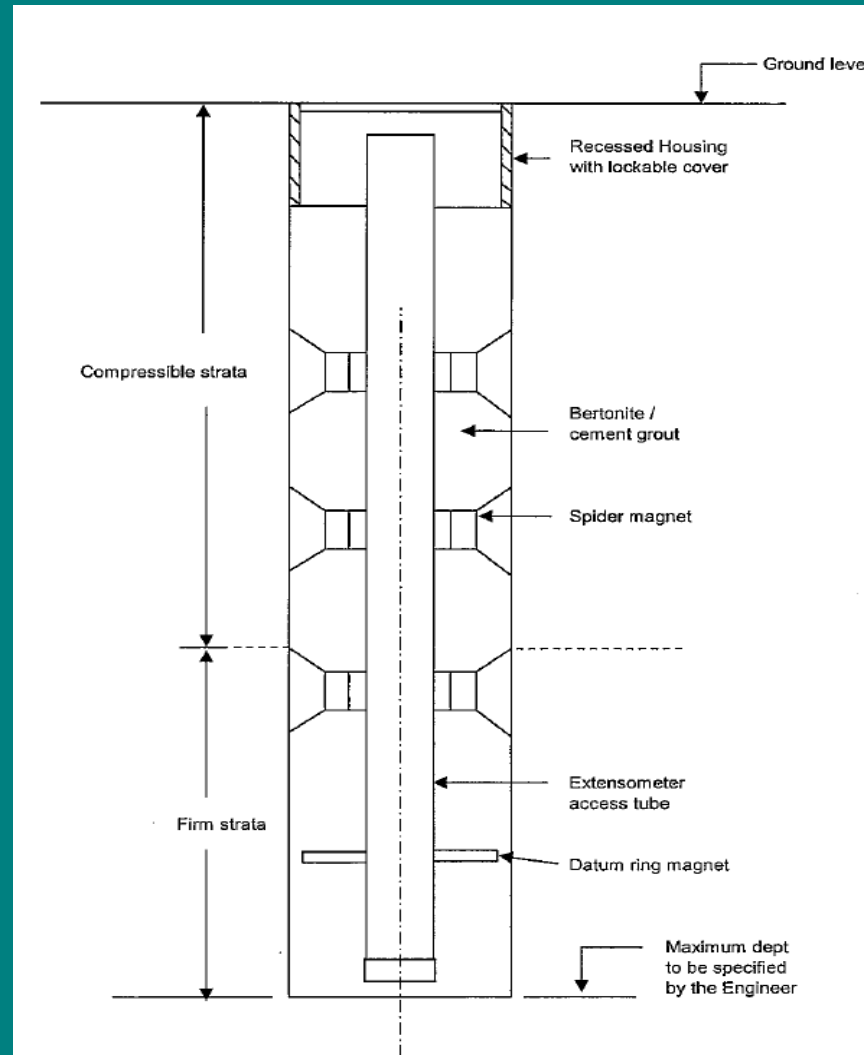
## Inclinometer



INCLINOMETER

# Instrumentation Details

## Magnetic Extensometer



**Surface  
Marker**

**Other Field  
Monitoring**

**Ground  
Marker**

**Visual  
Marker**

